

AD 740818



EXPERIMENTAL AND THEORETICAL  
STUDIES OF HULL-DECKHOUSE  
INTERACTION - 2-LEVEL  
DECKHOUSE

Reproduced from  
best available copy.



by

Johannes Buhler  
Lawrence J. Levy

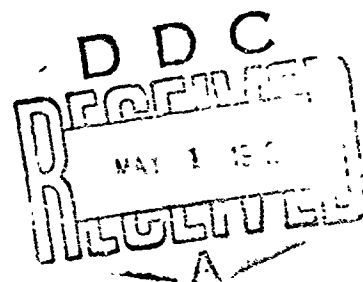
This research was sponsored by  
the Structures Department of the  
Naval Ship Research and Develop-  
ment Center under Naval Ship  
Systems Command Subproject SR  
011 0101, Task 0401, Contract  
N00014-67-A-0114-0006 and by  
Naval Ship Systems Command Con-  
tract N00024-67-C-5362.

This document has been approved  
for public release and sale; its  
distribution is unlimited.



Report No. NA-71-4  
August 1971

Reproduced by  
NATIONAL TECHNICAL  
INFORMATION SERVICE  
Springfield Va 22151



**COLLEGE OF ENGINEERING**  
**UNIVERSITY OF CALIFORNIA, Berkeley**

Reproduction in whole or in part is permitted for  
any purpose of the United States Government.

78

**Best  
Available  
Copy**

Unclassified  
Security Classification

DOCUMENT CONTROL DATA - R & D

(Security classification of title, body of abstract and indexing annotation must be entered when the overall report is classified)

|   |  |   |                       |
|---|--|---|-----------------------|
| 1. ORIGINATING ACTIVITY (Corporate author)<br>University of California<br>College of Engineering<br>Berkeley, California  |  | 2a. REPORT SECURITY CLASSIFICATION<br>Unclassified  |                       |
|   |  | 2b. GROUP   |                       |
| 3. REPORT TITLE<br><br>Experimental and Theoretical Studies of Hull-<br>Deckhouse Interaction - 2-Level Deckhouse   |  |   |                       |
| 4. DESCRIPTIVE NOTES (Type of report and inclusive dates)<br>Technical Report   |  |   |                       |
| 5. AUTHOR(S) (First name, middle initial, last name)<br><br>Johannes Buhler<br>Lawrence J. Levy   |  |   |                       |
| 6. REPORT DATE<br>August 1971   |  | 7a. TOTAL NO. OF PAGES<br>72 + iv   | 7b. NO. OF REFS<br>10 |
| 8a. CONTRACT OR GRANT NO.<br>N00024-67-C-5362 and<br>N00014-67-A-0114-0006<br>b. PROJECT NO.<br>SR-011-0101, Task 0401<br>c.<br>d.  |  | 9a. ORIGINATOR'S REPORT NUMBER(S)<br>College of Engineering<br>Report NA-71-4<br><br>9b. OTHER REPORT NO(S) (Any other numbers that may be assigned<br>this report) |                       |
| 10. DISTRIBUTION STATEMENT<br><br>This document has been approved for public release<br>and sale; its distribution is unlimited.  |  |   |                       |
| 11. SUPPLEMENTARY NOTES   |  | 12. SPONSORING MILITARY ACTIVITY  |                       |
| 13. ABSTRACT<br><br>The results of twelve tests of hull-deckhouse interaction are presented and discussed. The tests were carried out on the University of California Ship Structures Static Test Machine (Ref.[2]), using a 42-foot long longitudinally framed box girder with a 22-foot long two level deckhouse. This is a continuation of a previous report (Ref.[4]) in which tests were conducted using a single level deckhouse.<br><br>Deflections of hull and deckhouse were measured. Stress distributions along three transverse sections and along the deck to deckhouse joint were calculated from measured strains.<br><br>Three bending moment distributions and two deck support arrangements were used in the test program. The experimental results are compared with results from finite element analysis and two other theoretical methods of analysis of hull-deckhouse interaction. |  |   |                       |

Unclassified  
Security Classification

| 14. KEY WORDS   | LINK A |    | LINK B |    | LINK C |    |
|---|--------|----|--------|----|--------|----|
|   | ROLE   | WT | ROLE   | WT | ROLE   | WT |
| Ship Structures<br>Theoretical Analysis<br>Structural Experiments |        |    |        |    |        |    |

EXPERIMENTAL AND THEORETICAL STUDIES  
OF HULL-DECKHOUSE INTERACTION - 2-LEVEL DECKHOUSE

by

Johannes Buhler  
Lawrence J. Levy

This research was sponsored by the  
Structures Department of the Naval  
Ship Research and Development Center  
under Naval Ship Systems Command  
Subproject SR 011 0101, Task 0401,  
Contract N06014-67-A-0114-0006 and  
by Naval Ship Systems Command Contract  
N00024-67-C-5362.

This document has been approved for public  
release and sale; its distribution is unlimited.

Reproduction in whole or in part is permitted  
for any purpose of the United States Government.

College of Engineering  
University of California  
Berkeley, California

August 1971

# ABSTRACT

The results of twelve tests of hull-deckhouse interaction are presented and discussed. The tests were carried out on the University of California Ship Structures Static Test Machine (Ref.[2]), using a 42-foot long longitudinally framed box girder with a 22-foot long two level deckhouse. This is a continuation of a previous report (Ref.[4]) in which tests were conducted using a single level deckhouse.

Deflections of hull and deckhouse were measured. Stress distributions along three transverse sections and along the deck to deckhouse joint were calculated from measured strains.

Three bending moment distributions and two deck support arrangements were used in the test program. The experimental results are compared with results from finite element analysis and two other theoretical methods of analysis of hull-deckhouse interaction.

## TABLE OF CONTENTS

|   | <u>Page No.</u> |
|---|-----------------|
| Abstract  |                 |
| Table of Contents   | i               |
| List of Figures   | ii              |
| List of Tables  | iv              |
| I. Introduction   | 1               |
| II. Experimental Equipment and Methods  | 2               |
| III. Experimental Results   | 7               |
| IV. Finite Element Analysis   | 9               |
| V. Other Theoretical Methods  | 10              |
| VI. Discussion  | 12              |
| Figures   | 15-40           |
| Acknowledgements  | 41              |
| References  | 42              |
| Appendix A - Deflection Data  | 43              |
| Appendix B - Computer Programs  | 45              |
| Appendix C - Stresses and Strains   | 48              |
| Appendix D - Fourier Coefficients   | 60              |
| Appendix E - Computations for Comparison<br>of Kammerer's and Schade's<br>Methods of Deckhouse Analysis<br>with Experimental Data | 61              |

## LIST OF FIGURES

|   | <u>Page No.</u> |
|---|-----------------|
| 1 Midship Section of Structural Model                               | 15              |
| 2 Interior View of Hull Model                                       | 16              |
| 3 Deckhouse Construction Plan                                       | 17              |
| 4 Strain Gage Location Map - Deckhouse                              | 18              |
| 5 Strain Gage Location Map - Hull Girder                            | 19              |
| 6 Location of Dial Gages  | 20              |
| 7 Deckhouse Deflection Instrumentation                              | 21              |
| 8 Force Monitoring and Pressure Control                             | 22              |
| 9 Electronic Data Acquisition System                                | 23              |
| 10 Finite-Element Step Designation                                  | 24              |
| 11 Deflections and Longitudinal Stresses,<br>Test 8AH               | 25              |
| 12 Deflections and Longitudinal Stresses,<br>Test 9AH               | 26              |
| 13 Deflections and Longitudinal Stresses,<br>Test 8BH               | 27              |
| 14 Deflections and Longitudinal Stresses,<br>Test 9BH               | 28              |
| 15 Deflections and Longitudinal Stresses,<br>Test 8CH               | 29              |
| 16 Deflections and Longitudinal Stresses,<br>Test 9CH               | 30              |
| 17 Longitudinal Stress Distribution Amidships -<br>Stanchions Loose | 31              |
| 18 Longitudinal Stress Distribution Amidships -<br>Stanchions Fixed | 31              |
| 19 Shear and Vertical Stresses at Bond,<br>Tests 8A & 9A            | 32              |
| 20 Shear and Vertical Stresses at Bond,<br>Tests 8B & 9B            | 32              |
| 21 Shear and Vertical Stresses at Bond,<br>Tests 8C & 9C            | 33              |
| 22 Shear and Vertical Stresses at Deckhouse<br>End, Tests 8AH & 8AS | 34              |
| 23 Shear and Vertical Stresses at Deckhouse<br>End, Tests 9AH & 9AS | 34              |
| 24 Shear and Vertical Stresses at Deckhouse<br>End, Tests 8BH & 8BS | 35              |



## LIST OF FIGURES (cont.)

|  | <u>Page No.</u> |
|--|-----------------|
| 25 Shear and Vertical Stresses at Deckhouse End, Tests 9BH & 9BS                                   | 35              |
| 26 Shear and Vertical Stresses at Deckhouse End, Tests 8CH & 8CS                                   | 36              |
| 27 Shear and Vertical Stresses at Deckhouse End, Tests 9CH & 9CS                                   | 36              |
| 28 Comparison of Kammerer's and Schade's Analysis for a Two-Level Deckhouse Under Bending Moment B | 37              |
| 29 Comparison of Tests 9AH-S and Theory at Midships  | 37              |
| 30 Comparison of Tests 9AH-S and Theory at Midships  | 38              |
| 31 Comparison of Tests 8BH-S and Theory at Midships  | 38              |
| 32 Comparison of Tests 9BF-S and Theory at Midships  | 39              |
| 33 Comparison of Tests 8CH-S and Theory at Midships  | 39              |
| 34 Comparison of Tests 9CH-S and Theory at Midships  | 40              |

## LIST OF TABLES

|  | <u>Page No.</u> |
|--|-----------------|
| 1 Designation and Condition of Test                                  | 6               |
| 2 Pressure Distribution in Tests                                     | 6               |
| 3 Deflection Data, Appendix A  | 43              |
| 4 Corrected Deflection Data, Appendix A                              | 44              |
| 5 Stresses and Strains - Tests 8AH & 8AS,<br>Appendix C              | 48              |
| 6 Stresses and Strains - Tests 9AH & 9AS,<br>Appendix C              | 50              |
| 7 Stresses and Strains - Tests 8BH & 8BS,<br>Appendix C              | 52              |
| 8 Stresses and Strains - Tests 9BH & 9BS,<br>Appendix C              | 54              |
| 9 Stresses and Strains - Tests 8CH & 8CS,<br>Appendix C              | 56              |
| 10 Stresses and Strains - Tests 9CH & 9CS,<br>Appendix C             | 58              |
| 11 Fourier Coefficients for Experimental<br>Bending Moment Expansion | 60              |

## I. INTRODUCTION

This report presents the results of a series of twelve structural tests performed to study hull-deckhouse interaction. The model, testing apparatus, instrumentation, and testing method are detailed in Section II. The previous series of tests (Ref. [4]) were performed using a single deckhouse, whereas this series of tests was conducted using a two level deckhouse.

The presentation of the experimental data and the computation of the stresses are outlined in Section III.

The finite element analysis is outlined in Section IV.

Two other theoretical methods of stress analysis are described in Section V.

In Section VI, the experimental results are discussed. Also, the different theoretical methods of analysis are evaluated in this section.

## II. EXPERIMENTAL EQUIPMENT AND METHODS

### 1. Structural Model

A detailed description of the ship structural model is given in Ref.[4]. It is a longitudinally framed box girder, 42 feet long, 8 feet wide and 56 inches deep, with an attached 22-foot long deckhouse. For these tests the model was modified by adding a second level to the deckhouse.

A transverse section in way of the deckhouse is shown in Figure 1. A photograph of the interior of the hull model is shown in Figure 2. The model has no conventional transverse bulkheads. The restraint to vertical deformation of the main deck provided by such bulkheads is simulated by a number of stanchions attached to the deck, along the bond between deckhouse side and deck, and extending to the bilge of the model. When it is desired to make a stanchion effective, a spacer is inserted between the overlapping portion of the flanges of the upper and lower parts of the stanchion and the two are firmly bolted together.

The model deckhouse construction plan is shown as Figure 3. The two decks of the two-level deckhouse are bolted to the sides and ends of the deckhouse with 1/4-inch bolts on 3-inch centers. All other structural joints are welded. The diaphragm plate, shown in Figure 3, was installed on one end only. At the other end, the deckhouse was welded directly to the upper deck of the hull girder.

### 2. Load Application

The test model was subjected to static loads on the Ship Structures Static Testing Machine which is located at the University of California Richmond Field Station and described in Ref.[2]. The model rests on seven bags of equal size which cover its entire bottom area. Each bag has its individual control

valve and can be subjected to air pressures ranging from +12 to -12 psig; this feature allows application of positive and negative loads to each of seven transverse sections of the test model under investigation. When not under tests, the model is suspended at its four corners by means of turnbuckles. During the entire test the pressure in two of the seven bags is adjusted to carry the weight of the model, so that the forces in the turnbuckles decrease to zero. This is continuously checked by recording the forces on a 4-track Sanborn, Model 154, strip chart recorder (Figure 8).

### 3. Strain Gage Instrumentation

A strain gage map showing the location of the 524 gages applied to the model is shown in Figures 4 and 5. Figure 4 is an expanded view of the deckhouse and the first level deck of the deckhouse. Figure 5 is an expanded view of the hull girder. Gages are distributed along three transverse sections of the hull and deckhouse, and along the joint of the deckhouse sides and ends to the main deck.

Gages are generally applied to both sides of the plate in order to obtain the "heart-of-plate" strain. All gages are of etched foil type, with a resistance of 120  $\Omega$ . Three different sizes of these gages have been selected to suit the requirements at different locations. Those used for all 3-gage rosettes were 1/4" x 1/8". In positions 713 to 721, ready-made rosettes with 1/8" x 1/16" gages were installed. In all other locations, 1/2" x 1/4" gages were used. The strain gages were connected to bridge completion boxes (Figure 7). Each box has thirty-six terminals. Thirty-five terminals were used for active gages, the remaining terminal was connected to a dummy gage (resistor). The strains were measured automatically in an electronic data acquisition system (Figure 9) and recorded on magnetic tape. A description of the electronic system is given in Ref.[1].

#### 4. Deflection Instrumentation

A total of seventeen dial gages were used to measure the relative deflection of hull and deckhouse. The distribution of these gages is shown in Figure 6. All gages were attached to two trusses running along the outer edges of the main deck and supported only near the ends of the hull. Gages lettered A to K were used to measure the deflection of the hull along the deck edge. Gages numbered 1 to 6 gave the deflection of the deckhouse as measured at the edge of the deckhouse deck. Gages 5, 6, F, I and J were installed to check the symmetry of the deflections about both axes of the model. Some of the gages can be seen in Figure 7, which is an end view of the deckhouse, looking aft.

The lowest resolution reading of the deflection gages was 0.001 inch.

#### 5. Test Conditions

Load and stanchion support conditions of the twelve tests discussed in this report are summarized in Table 1. The last letter in the designation given to each test (S or H) represents sagging or hogging. Net pressures applied by each of the bags are listed in Table 2. The odd values of pressure result from setting the pressure regulators in units of cm. of Hg rather than psi.

#### 6. Test Methods

Each experiment consisted of reading strains and deflections at each of three load conditions. The first and third load conditions, designated initial and final, have the model afloat on bags 3 and 5. The second, or test step, has the model under pressures corresponding to those listed in Table 2. Bags 3 and 5 have an additional pressure sufficient to float the loaded model. In general the pressure in bags 3 and 5 is approximately the sum of the pressure in either the initial and final step and the nominal

test pressure. Any deviation is due to pull on the model periphery by the strap sealing the vacuum surrounding the pressure bags. A detailed description of the test procedure is given in Ref.[1].

TABLE 1 DESIGNATION AND CONDITION OF TEST

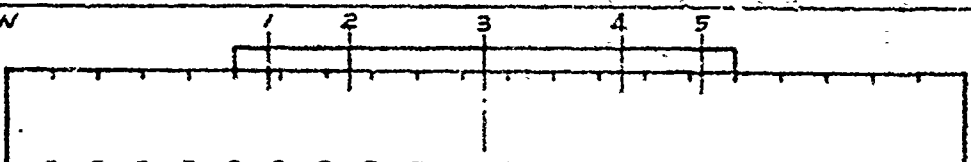



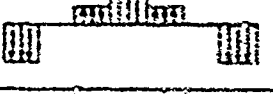


| TEST SECTION                  |  |   |   |   |   |   |   |   |   |   |   |     |    |    |    |    |    |    |    |    |    |    |    |
|-------------------------------|--|---|---|---|---|---|---|---|---|---|---|-----|----|----|----|----|----|----|----|----|----|----|----|
| FRAME NO.                     |  | 0   | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10  | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 |
| STANCHIONS<br>AT<br>FRAME NOS |  | NONE  |   |   |   |   |   |   |   |   |   | ALL |    |    |    |    |    |    |    |    |    |    |    |
| LOAD<br>DISTRIBUTION          |  |   |   |   |   |   |   |   |   |   |   | 8AS |    |    |    |    |    |    |    |    |    |    |    |
|                               |  |  |   |   |   |   |   |   |   |   |   | 3AH |    |    |    |    |    |    |    |    |    |    |    |
|                               |  |  |   |   |   |   |   |   |   |   |   | 8BS |    |    |    |    |    |    |    |    |    |    |    |
|                               |  |  |   |   |   |   |   |   |   |   |   | 3BH |    |    |    |    |    |    |    |    |    |    |    |
|                               |  |  |   |   |   |   |   |   |   |   |   | 8CS |    |    |    |    |    |    |    |    |    |    |    |
|                               |  |  |   |   |   |   |   |   |   |   |   | 3CH |    |    |    |    |    |    |    |    |    |    |    |

TABLE 2 Pressure Distribution in Tests

| Pressure in Bag No.<br>(psig) |       |        |        |        |       |       | Test No. |
|-------------------------------|-------|--------|--------|--------|-------|-------|----------|
| 1                             | 2     | 3      | 4      | 5      | 6     | 7     |          |
| 7.73                          | 0     | -7.73  | 0      | -7.73  | 0     | 7.73  | 319 AS   |
| -7.73                         | 0     | 7.73   | 0      | 7.73   | 0     | -7.73 | 319 AH   |
| 7.73                          | 0     | -3.865 | -7.73  | -3.865 | 0     | 7.73  | 319 BS   |
| -7.73                         | 0     | 3.865  | 7.73   | 3.865  | 0     | -7.73 | 319 BH   |
| 0                             | 5.03  | 10.06  | 10.06  | 10.06  | 5.03  | 0     | 319 CS   |
| 0                             | -5.03 | 10.06  | -10.06 | 10.06  | -5.03 | 0     | 319 CH   |



### III. EXPERIMENTAL RESULTS

#### 1. Presentation of Deflection Data

As shown in Figure 6, dial gages were distributed port and starboard on both the main deck edge and the deckhouse edge. The gages on the starboard side were installed to check the symmetry of the deflections about the longitudinal axis of the model. The maximum difference between port and starboard readings was  $4 \cdot 10^{-3}$  in. The maximum difference between deflection readings for hogging and sagging was  $3 \cdot 10^{-3}$  in. For these reasons, only port deflections in hogging were plotted.

Deflection readings are shown in Appendix A. A correction had to be made on the deflection readings because the trusses were supported at 18" from the ends of the main deck and not at its ends. The correction was obtained by extrapolation of the deflection curve to the end of the deck (see Ref.[1]). Corrected values are listed in Table 4 and plotted in Figures 11-16.

#### 2. Computation of Strains and Stresses

The strain data are stored on magnetic tape in binary coded decimal form. During one test of three runs, 1572 values are obtained from 524 strain gages. Each value, called a sample, consists of 6 digits representing a number from 0 to 12000 and a sign. Two computer programs, reproduced in Appendix B, are used to process the strain readings.

A program DATDUMP serves to read the data from tape and to punch them on card in a convenient form. Each card contains the initial, test, and final readings (runs) for two strain gages. Simultaneously, a printout of the data is made.

A second program, called STRESS serves to read and process the data obtained with DATDUMP. Bad data with non-interpretable

digits are changed to zero and listed for identification. The readings are then converted into strain values by multiplication with a calibration factor (Statement 3010). "I" is an index assigned to each strain gage consistent with the chronological order in which the gages are scanned. The strain readings  $X(I)$  are then assembled for each position (Statements 14 through 742). Note that the position numbers are identical with the statement numbers. The designations  $A_1$  through  $A_6$  refer to the direction and location of the gages according to the comment cards following statement 2400. For a definition of longitudinal, transverse and diagonal directions, see Appendix B. The values of  $A_1$  to  $A_6$  are printed in the last six columns of Tables 5 to 10.

Stresses are computed for each position number in Statements 2501 to 2505, assuming the following values for the material constants:

$$E = 3 \times 10^6 \text{ lbs/in}^2$$

Modulus of elasticity

$$G = 11 \times 10^6 \text{ lbs/in}^2$$

Shear modulus

$$\nu = 0.3$$

Poisson's constant

The stress values are printed out in the first 5 columns of Tables 5 to 10. Some values are plotted in Figures 11 to 27.

#### IV. FINITE ELEMENT ANALYSIS

The finite element analysis is based on procedures in which the real continuous structure is approximated by an assemblage of simple structural elements. Conditions of equilibrium of forces and compatibility of displacements are required to be satisfied at discrete locations throughout the approximating structure. This results in the reduction of the problem to the solving of sets of simultaneous equations relating these forces and displacements.

The degree of approximation involved varies, depending on the size of the mesh used in subdividing the structure into elements, but satisfactory convergence to the exact solution is generally obtained if the mesh size is made sufficiently small. One should see Ref.[7] for a more complete description of this type of analysis.

It is necessary to use a very fine mesh when examining regions of high stress concentration, such as the connection of a deckhouse end structure to the hull, resulting in excessive demands on computer capacity. A procedure is outlined in Ref.[7] whereby one may examine closely, by a succession of refinements, small regions of interest in a large structure.

In the analysis used here, one-quarter of the deckhouse (cut along centerline & midships) was used in step one. These results were plotted against the experimental data at the three cross sections, Figures 11-18. In the vicinity of the deckhouse end, two refinements were made in the size of the segment examined, and the results of step three were plotted against the experimental data, Figures 22-27.

The boundaries used in the different steps are shown in Figure 10.

## V. OTHER THEORETICAL METHODS

Two other more simplified methods of analysis were used to analyze the stresses at the midship section. The first one was proposed by Kammerer, Ref.[6], and the second one was proposed by Schade, Ref.[10].

Kammerer's method for calculating the stress distribution across hull and deckhouse utilizes semi-empirical results of fullscale experiments to evaluate the effect of differential deflections between deckhouses and their hull girders. These data have been incorporated into an analytical treatment of the problem based on plane stress theory. Shear lag is taken into account in figuring the "equivalent area" of the deckhouses, but is not considered in the hull girder analysis. The design charts given in Kammerer's paper are plotted using full ship dimensions, and it was therefore necessary to extrapolate model scantlings to full-scale. Calculations for one case are shown in Appendix E.

Schade's method for calculating the stress distribution across hull and deckhouse is also based on plane stress theory, but differs from Kammerer's in that shear displacement is accounted for in the sides of the hull and deckhouse and shear lag in the hull girder is included. The last item allows the stress at the deckhouse-hull connection to be different from the stress at the hull side. This method does not depend on empirical data, but instead upon the evaluation of a deck flexibility factor "K". The values of "K" were determined experimentally in Ref.[3] to be 650 psi for the all stanchions loose condition and 34,000 psi for the all stanchions fixed condition.

Fourier coefficients for the expansion of the three bending moments used in the experiments are listed in Appendix D.

The nomenclature for Schade's method is reproduced in Appendix E, along with calculations for one case.

Both of these methods were based on the simplifying assumption that the bending moment could be expanded into one constant term and one sinusoidal term. This is a good assumption in most cases, but will not hold in the case of bending moment "C", a "saddle form" bending moment. It was therefore necessary to return to the basic formulation of the equations and rederive them based on "n" sinusoidal terms. This was done for Schade's methods using Reference [9]. The new equations are shown at the end of Appendix E.

First a new equation for "r" was developed and is shown as Eq. E-8A. Next, using the new assumption for bending moment (Eq. E-9A), a new equation for " $p_f$ " was obtained and is shown as Eq. E-10A. The first four terms of Eq. E-10A are from the homogeneous solution and the last two terms are from the particular solution of the differential equation. The first four terms have coefficients which must be evaluated using the boundary conditions. All six terms must be evaluated for each term of the moment expansion. Because "p" will be different for each term of the moment expansion, all of the "effective" geometric properties, and coefficients calculated from these, have to be recalculated for each term also. Figures 33 and 34 were obtained using a three term expansion.

## VI. DISCUSSION

As stated, this report is a continuation of a previous report (Ref.[4]). The main reason for conducting this series of tests was to find what effect the addition of a two-level deckhouse would have on the previous experimental results. Another reason was to further check the various theoretical methods of predicting stress. Only the two extreme stanchion support conditions were used (no stanchions, and all stanchions fixed). Three bending moment distributions were used, instead of five as in the previous work, since the previous variation in moment distribution gave no significant variation in longitudinal direct stress distribution (except for "saddle-form" bending moment "C").

The experimental results followed the same basic form as shown in Part I (Ref.[4]), the only major difference being the reversal of stress in the deckhouse in the "no stanchion" condition. Where, in the one-level deckhouse experiments, the stress would decrease as one moves from the hull girder to the deckhouse top, now the stress decreases and reverses sign below the deckhouse top. This could have been expected from extrapolation of the previous results, but goes against intuition.

In Figures 11-16, the experimental longitudinal stresses and deflections are plotted along with the theoretical finite element values. The two agree very well, although the finite element deflections and stresses are generally slightly less than experimental values for moment distributions "A" and "B", and slightly greater than experimental values for moment distribution "C".

In Figures 17 and 18, the experimental longitudinal stress distributions amidships are compared. They are basically the same as shown in Part I (Ref.[4]), except that the second level of the deckhouse has lessened the effect of moment distribution on deckhouse stresses. The plots were obtained by multiplying the

computed stresses for each moment distribution by the ratio of the midship value of bending moment "C" to the midship value of their respective bending moments.

Figures 19-21 are again similar to those of Part I. These figures show the shear and vertical stresses at the deck-house-hull bond. They show, as expected, highest vertical stresses at the deckhouse end and over the midship stanchions and frames (higher values when stanchions are in place). They show high shear stress at the deckhouse end, continuously decreasing to zero at amidships.

Figures 22-27 compare the experimental values of vertical and shear stress with the theoretical values of the third step finite element analysis. There is generally good agreement between the two values, especially for the vertical stress. These figures again show the difficulty of obtaining accurate values at the corners, due to the finite size of the strain gages. Because of the very steep stress gradient, it is impossible to measure the maximum stresses involved.

Figure 28 shows a comparison of the values obtained by Kammerer's and Schade's methods for a typical bending moment. This shows that Kammerer's method will give results between the two extreme values of Schade's method, but that it gives results much closer to the rigid deck extreme than to the flexible extreme. This should be expected since most naval and passenger ships upon which the empirical data are based have fairly rigid main deck support. For this reason, in Figures 29-34, Kammerer's method is only compared with experiment for the "all stanchions fixed" condition.

In Figures 29-34, the experimental results at amidships are plotted along with Kammerer's and Schade's methods. The comparison is generally good. As stated in Section V, it was necessary to extend Schade's method in order to obtain reasonable correlation

between experimental and theoretical results for bending moment "C".

Kammerer's method will give a reasonably good approximation of stress when very little information is available on the ship's structure. It is by far the simplest method to apply, but requires that the bending moment be of simple sinusoidal shape.

Schade's method also gives a good approximation of stress, but requires the finding of a deck flexibility factor (which makes it more versatile). It is fairly simple to apply, unless more than one sinusoidal term is required to approximate the bending moment distribution, which complicates the problem somewhat.

The finite element method can, of course, be the most accurate, but also requires the greatest amount of work to solve a problem.



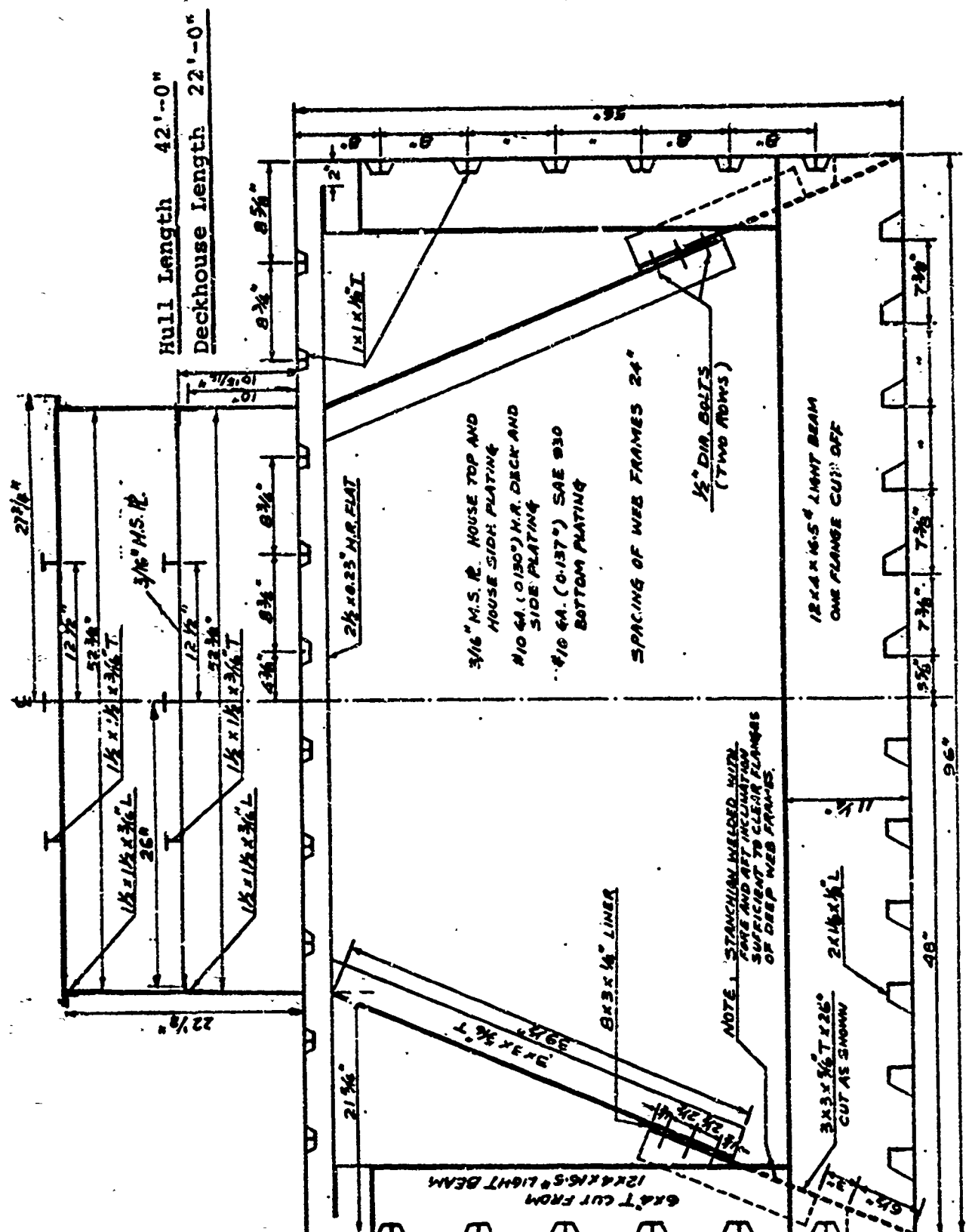
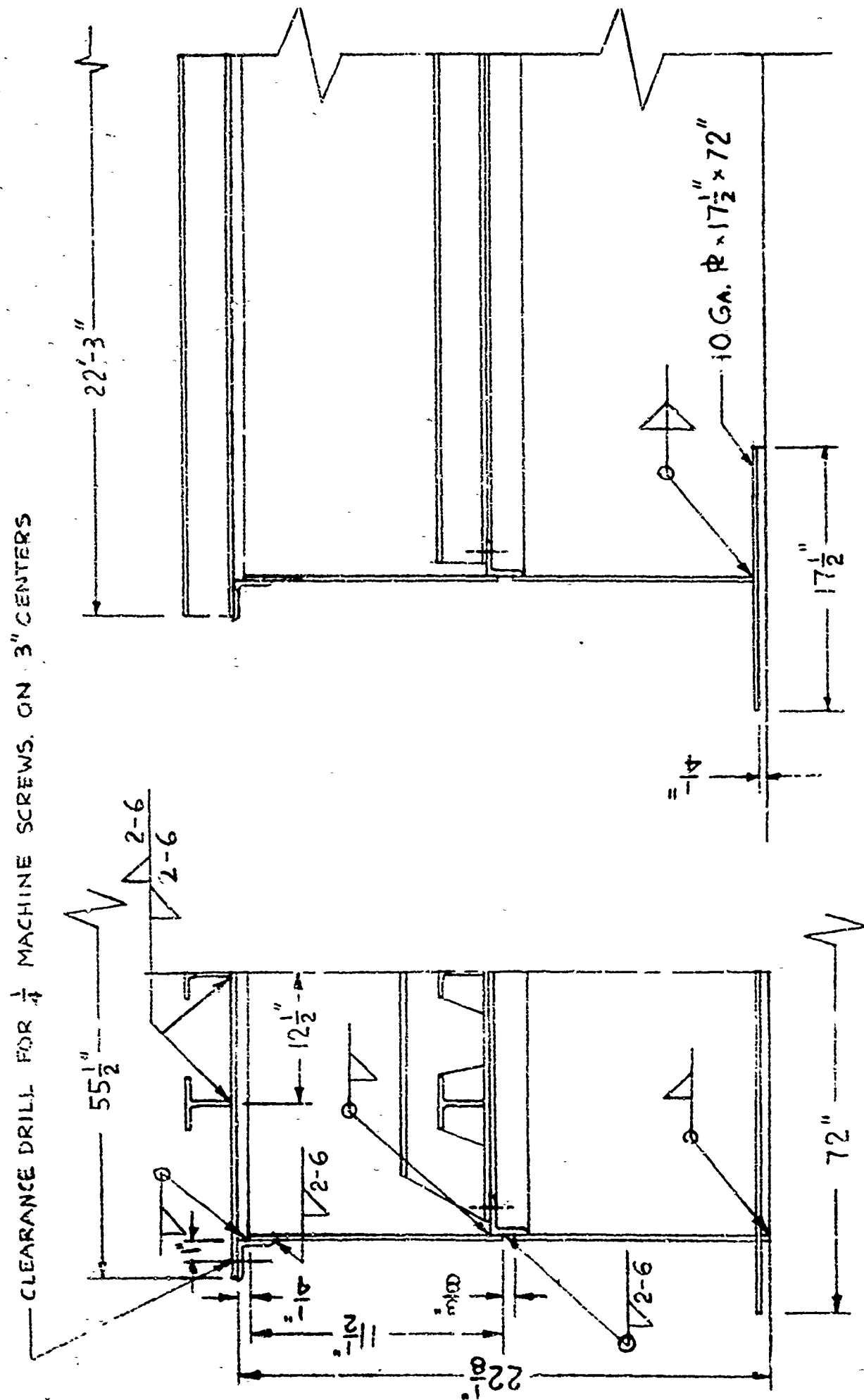


FIGURE 1. MIDSHIP SECTION OF STRUCTURAL MODEL



FIGURE 2. INTERIOR VIEW OF HULL MODEL



**FIGURE 3. DECKHOUSE CONSTRUCTION PLAN!**

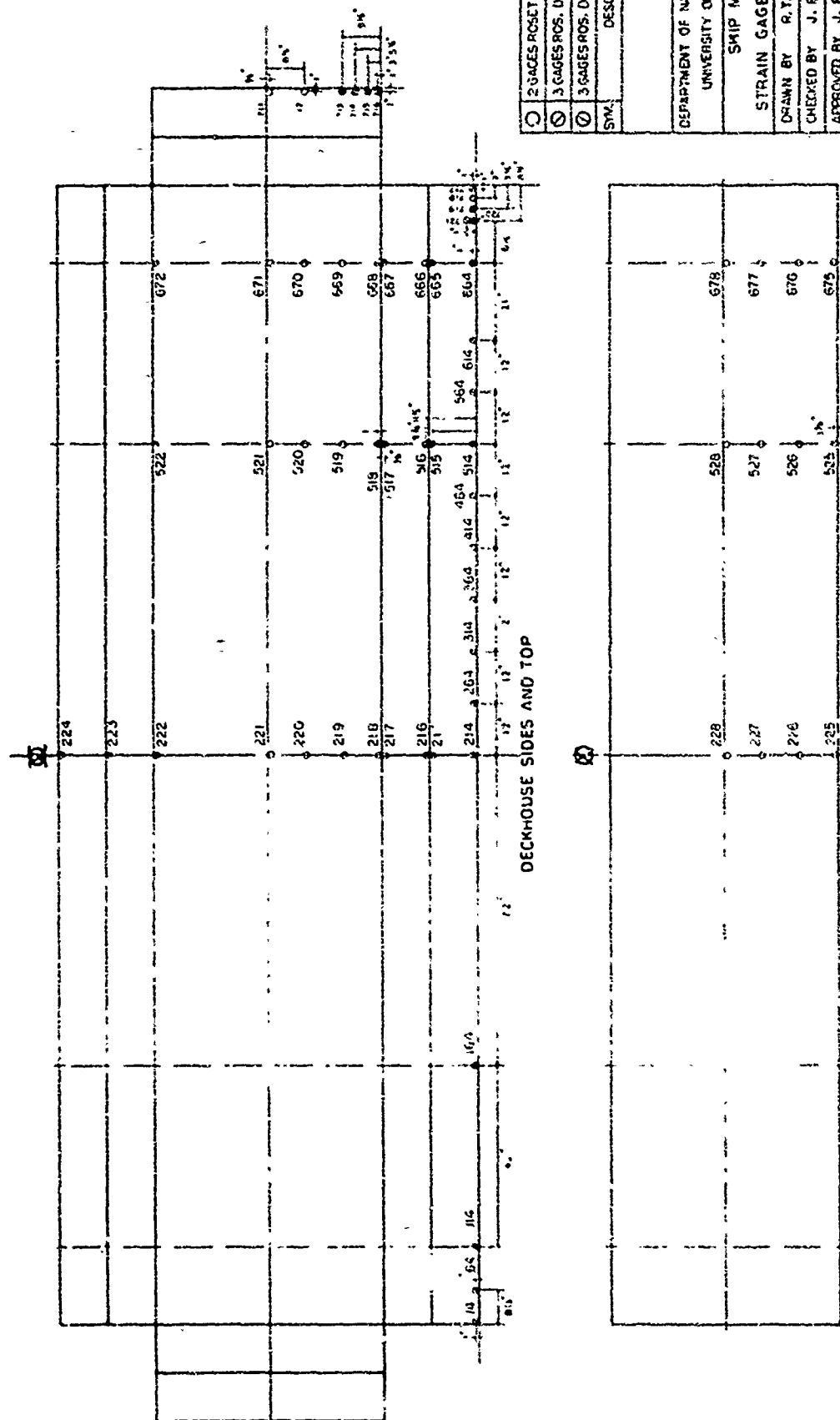


FIGURE 4. STRAIN GAGE LOCATION MAP - DECKHOUSE

|      |                               |
|------|-------------------------------|
| 2    | GAGES ROSETTE                 |
| 3    | GAGES POS. DIAG. IN C. TON    |
| 3    | GAGES POS. DIAG. IN DIRECTION |
| SUM. | DESCRIPTION                   |

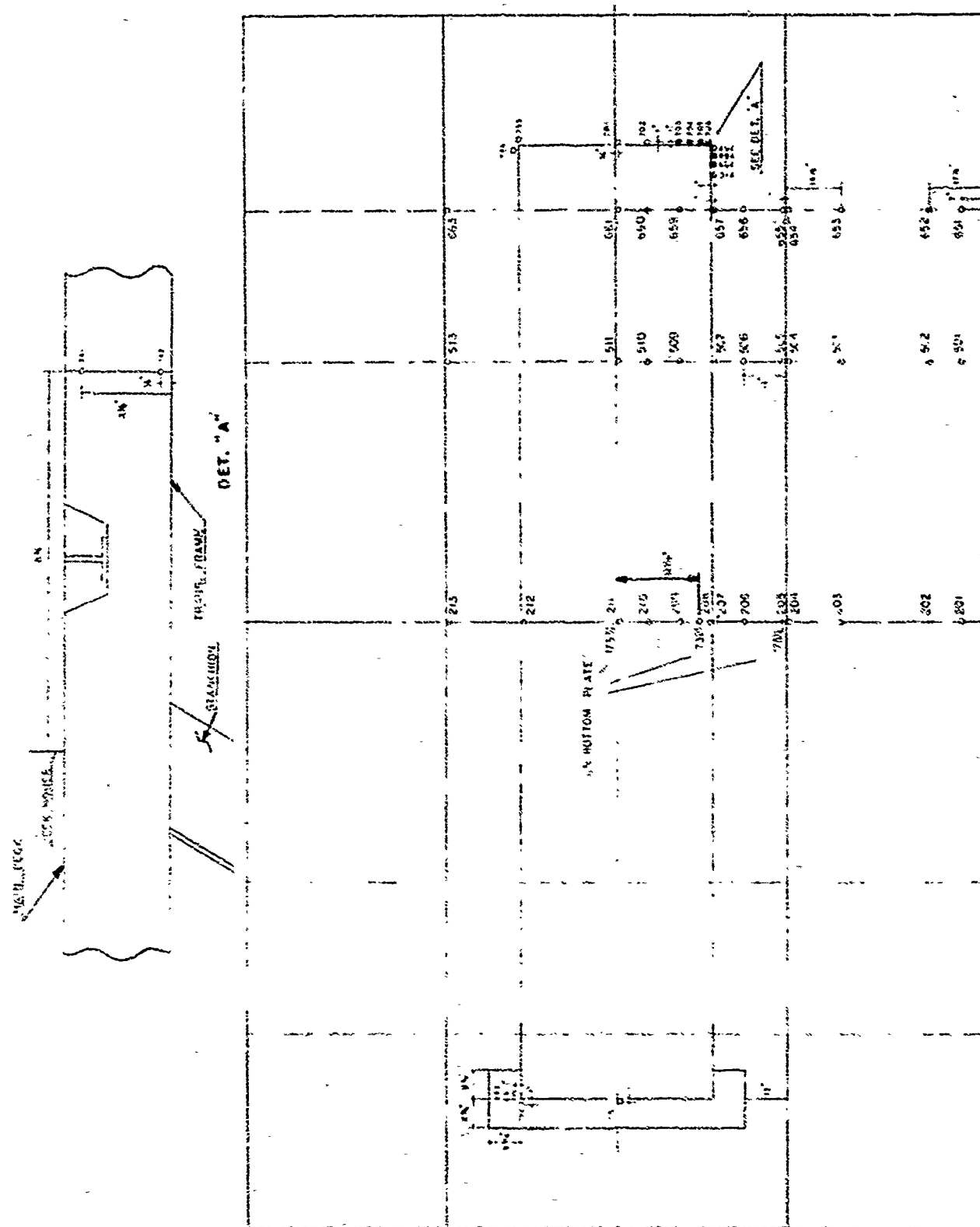
DEPARTMENT OF NAVAL ARCHITECTURE  
UNIVERSITY OF CALIFORNIA

SHIP MODEL

STRAIN GAGE POSITIONS

DRAWN BY R. T. HUANG  
CHECKED BY J. BUMLER  
APPROVED BY J. BUMLER

DWG. NO. TON-01 DATE JULY 1969



HULL GIRDER SIDES & TOP

FIGURE 5. STRAIN GAGE LOCATION MAP - HULL GIRDER

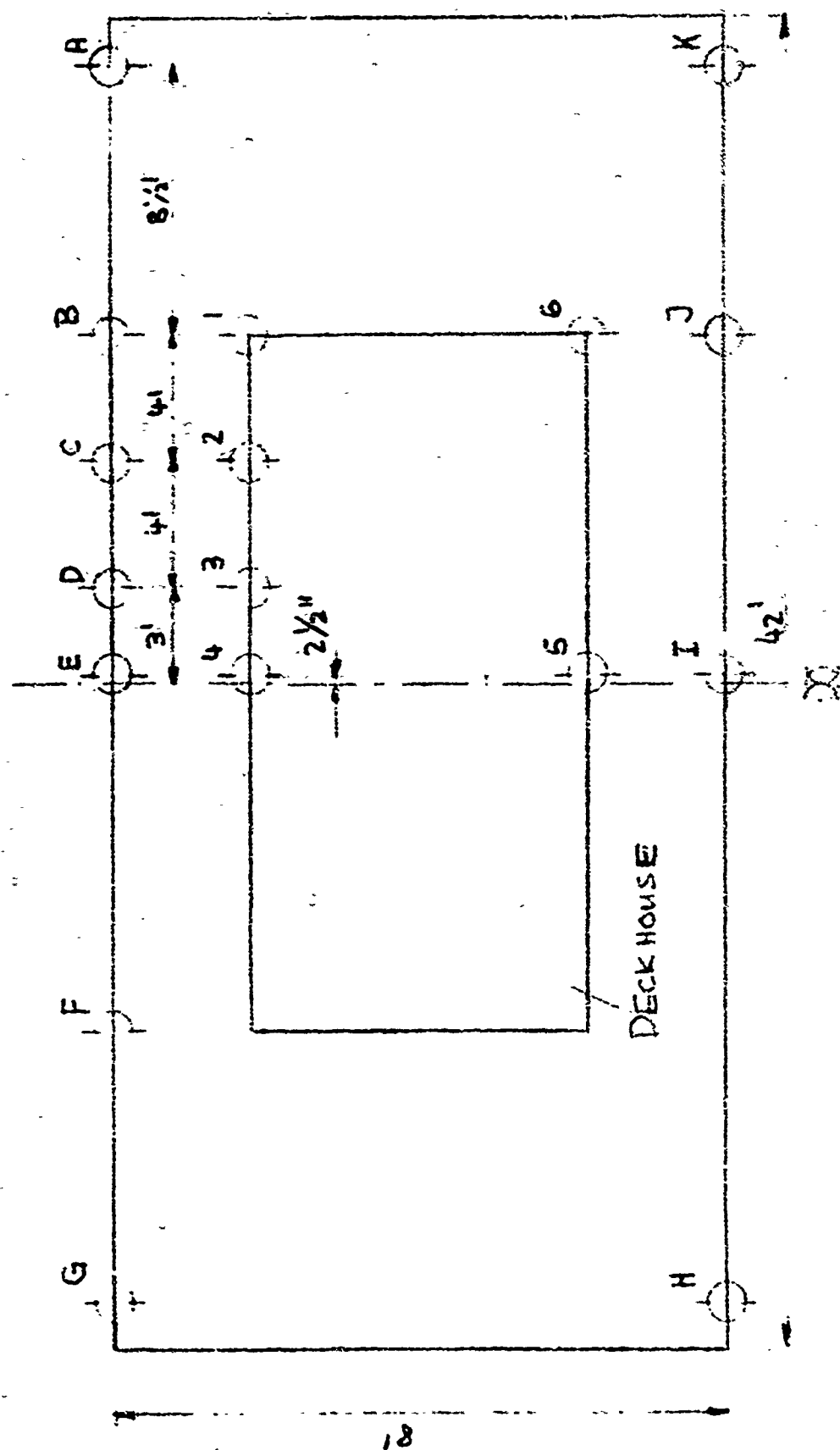


FIGURE 6. LOCATION OF DIAL GAGES

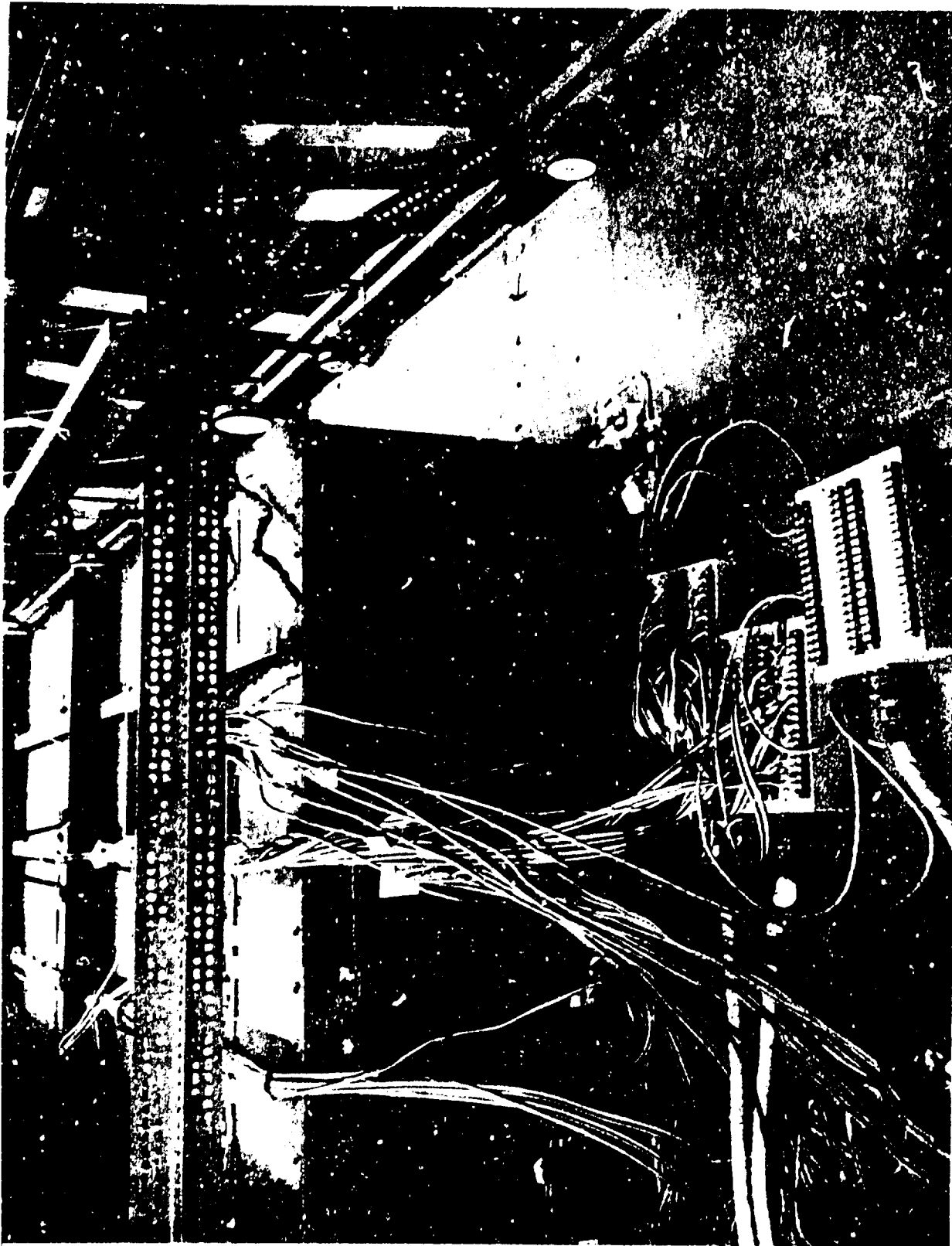


FIGURE 7. DECKHOUSE DEFLECTION INSTRUMENTATION



FIGURE 8 FORCE MONITORING AND PRESSURE CONTROL





FIGURE 9 ELECTRONIC DATA ACQUISITION SYSTEM

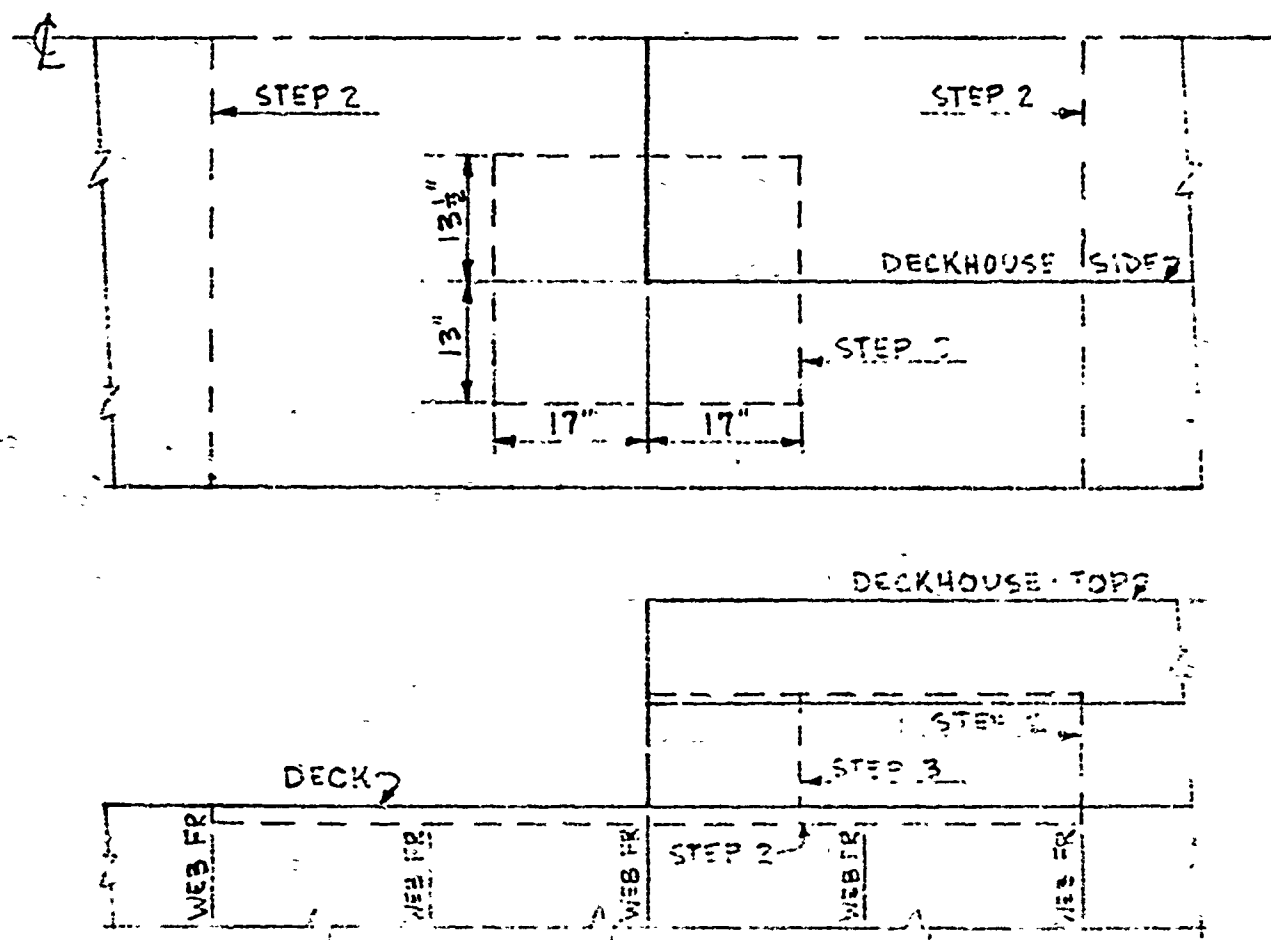


FIGURE 10. FINITE-ELEMENT STEP DESIGNATION

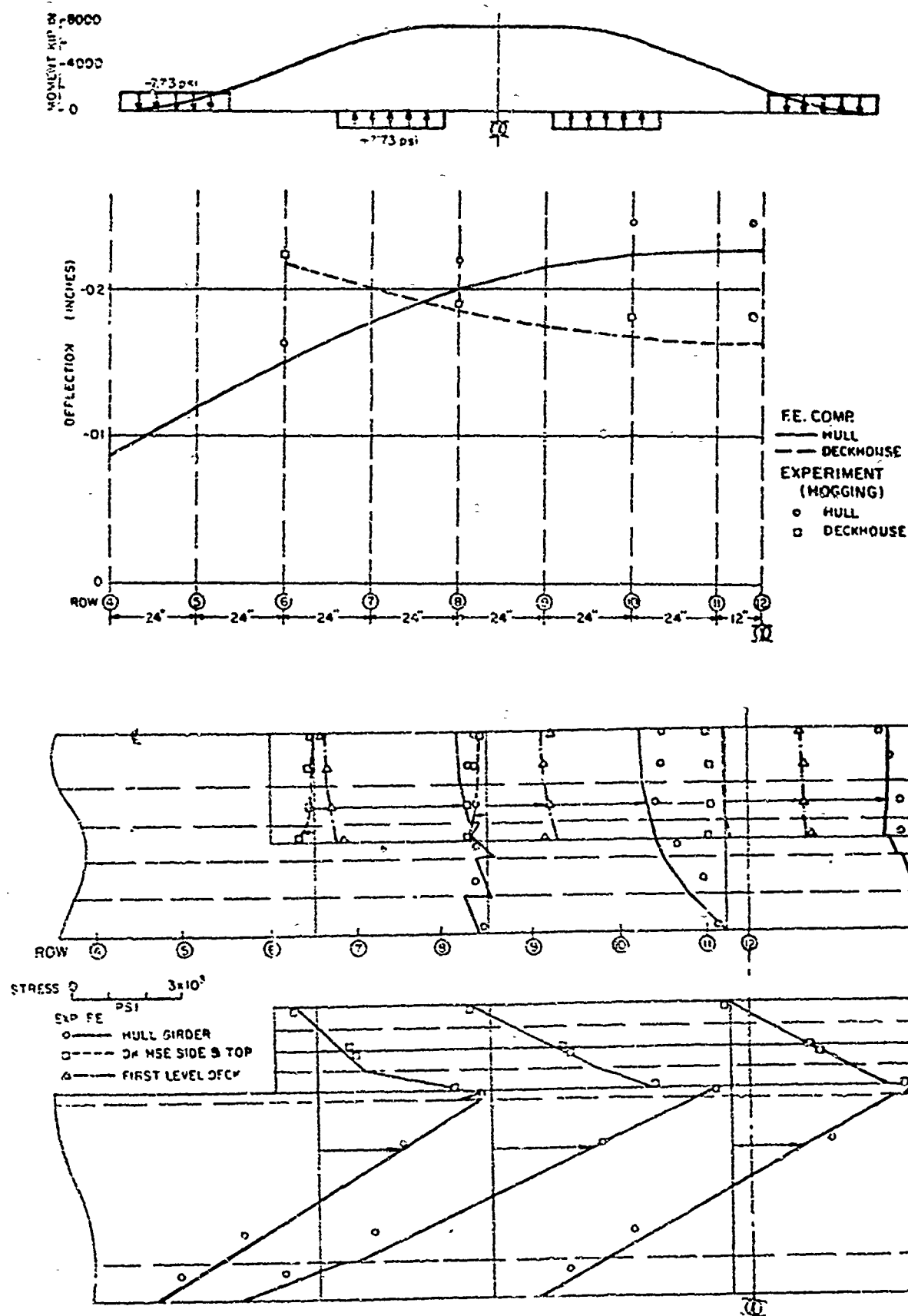


FIGURE 11. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 8AH

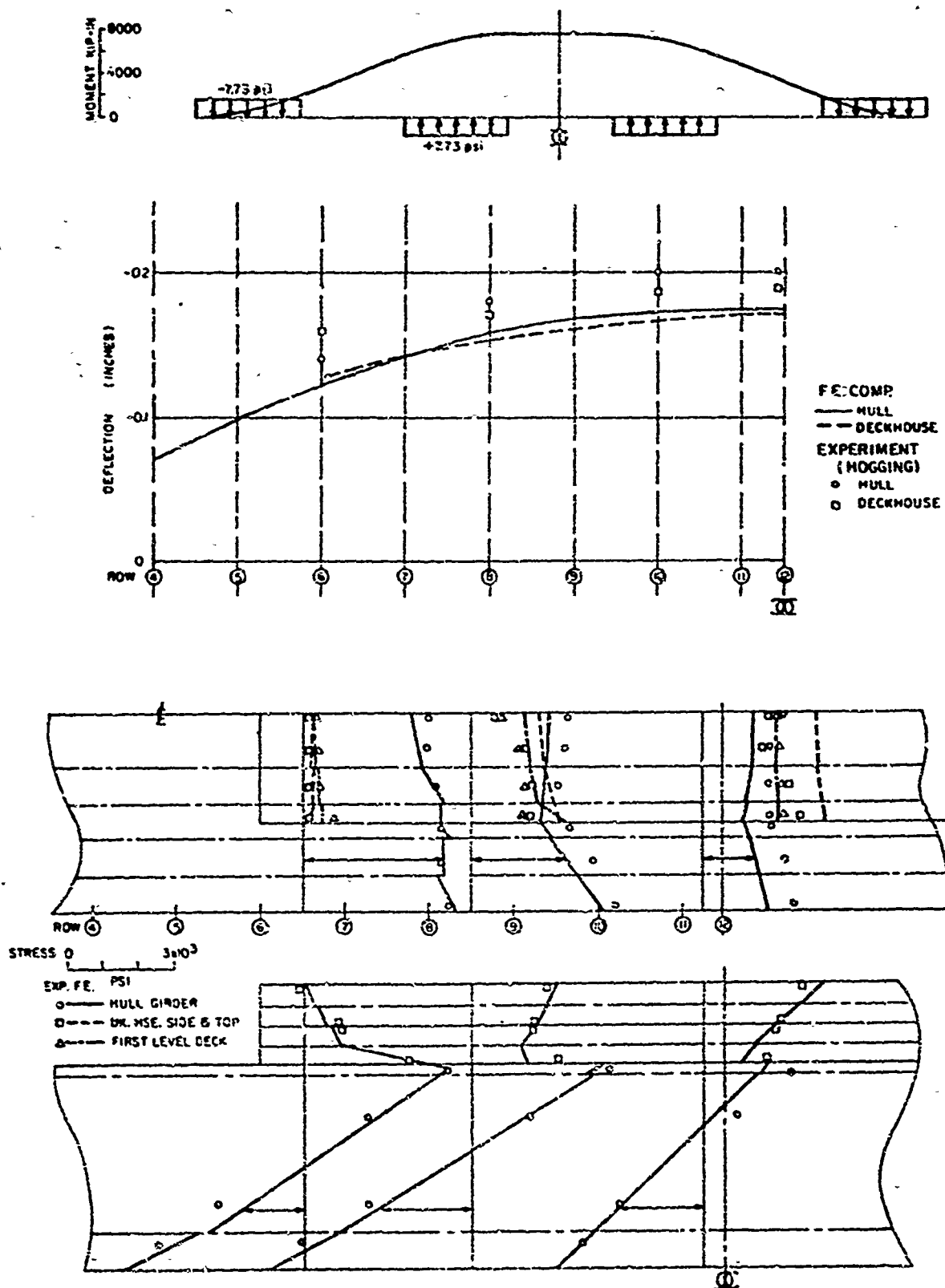


FIGURE 12. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 9AH

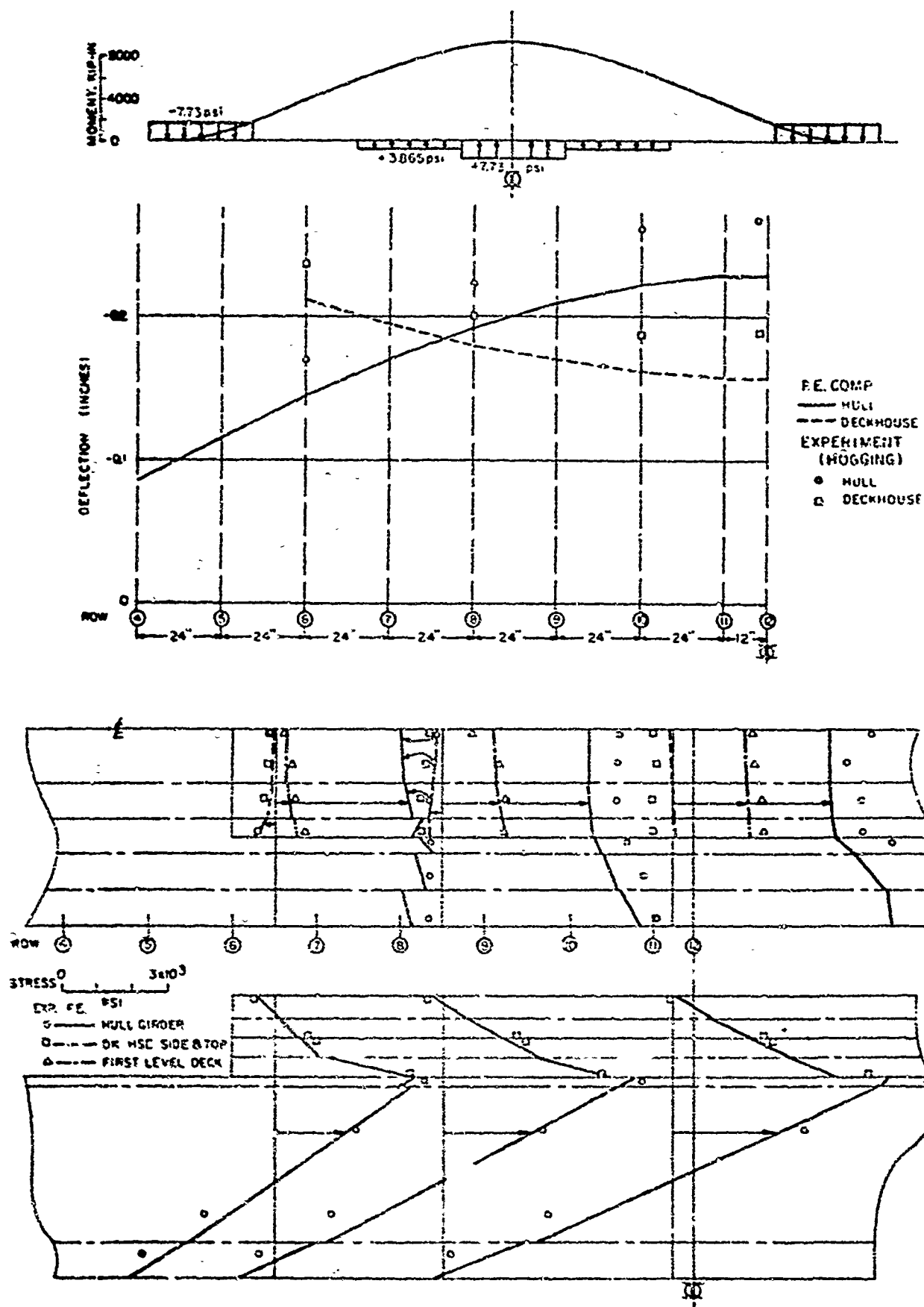


FIGURE 13. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 8BH

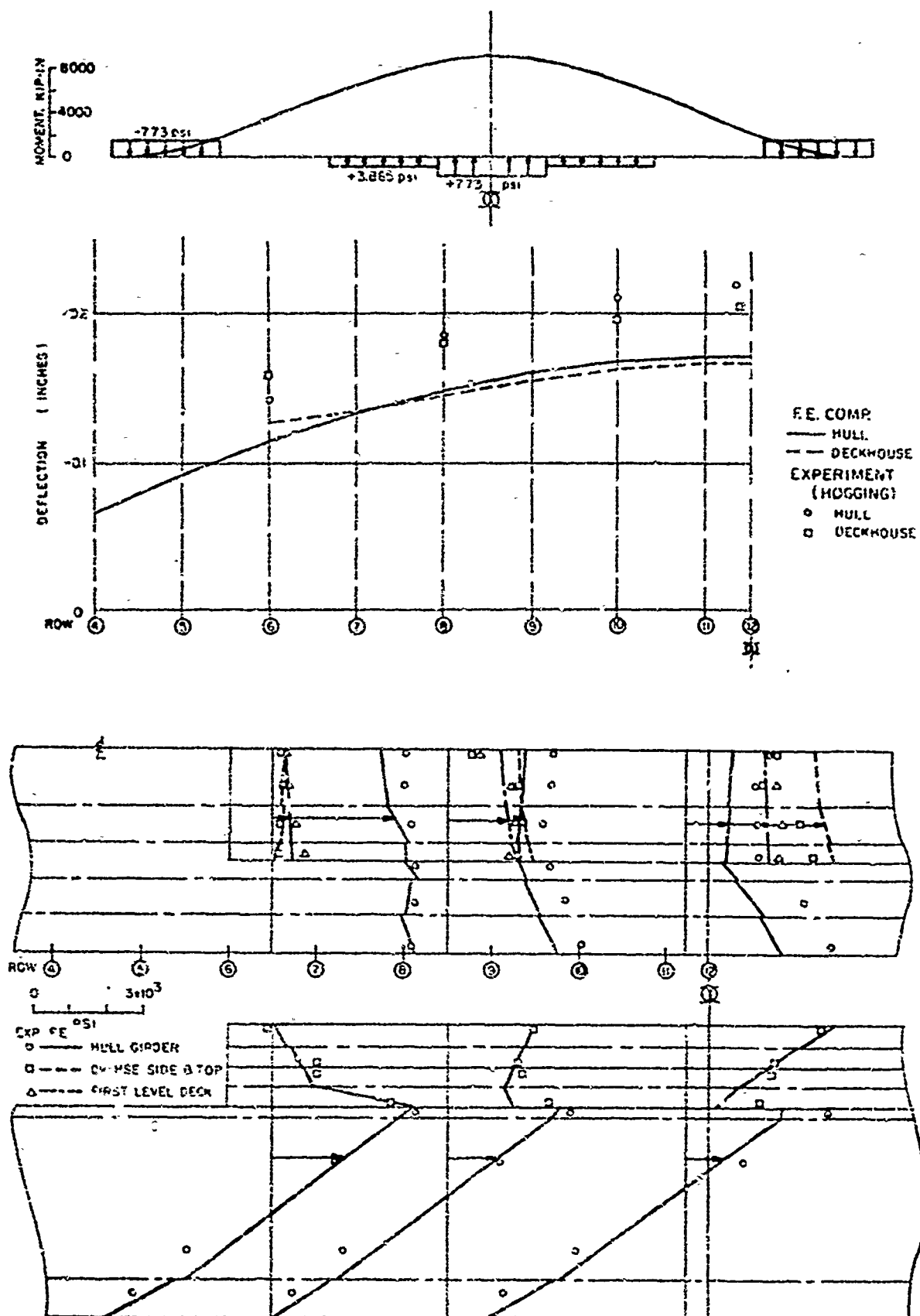


FIGURE 14. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 9BH

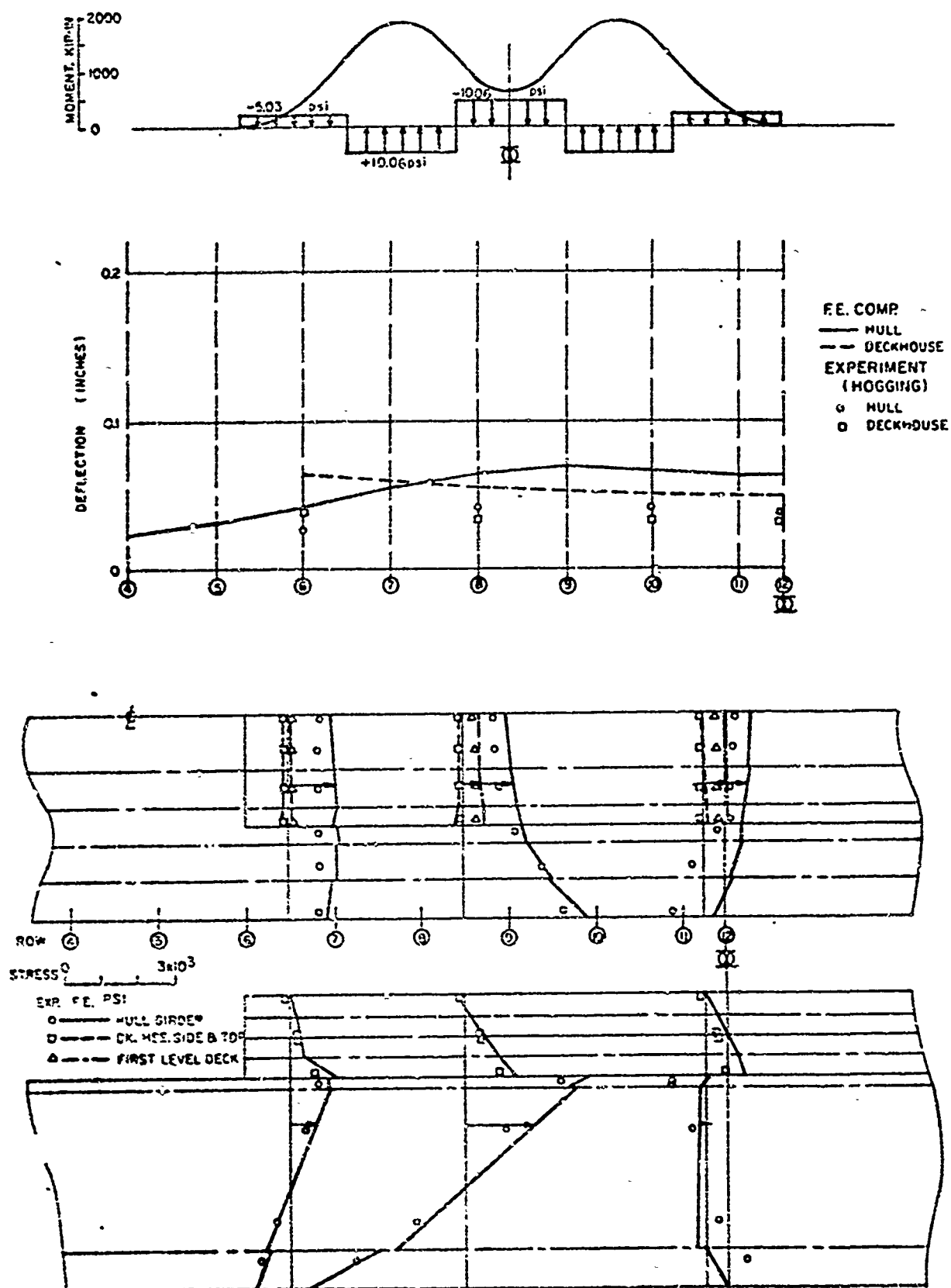


FIGURE 15. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 8CH

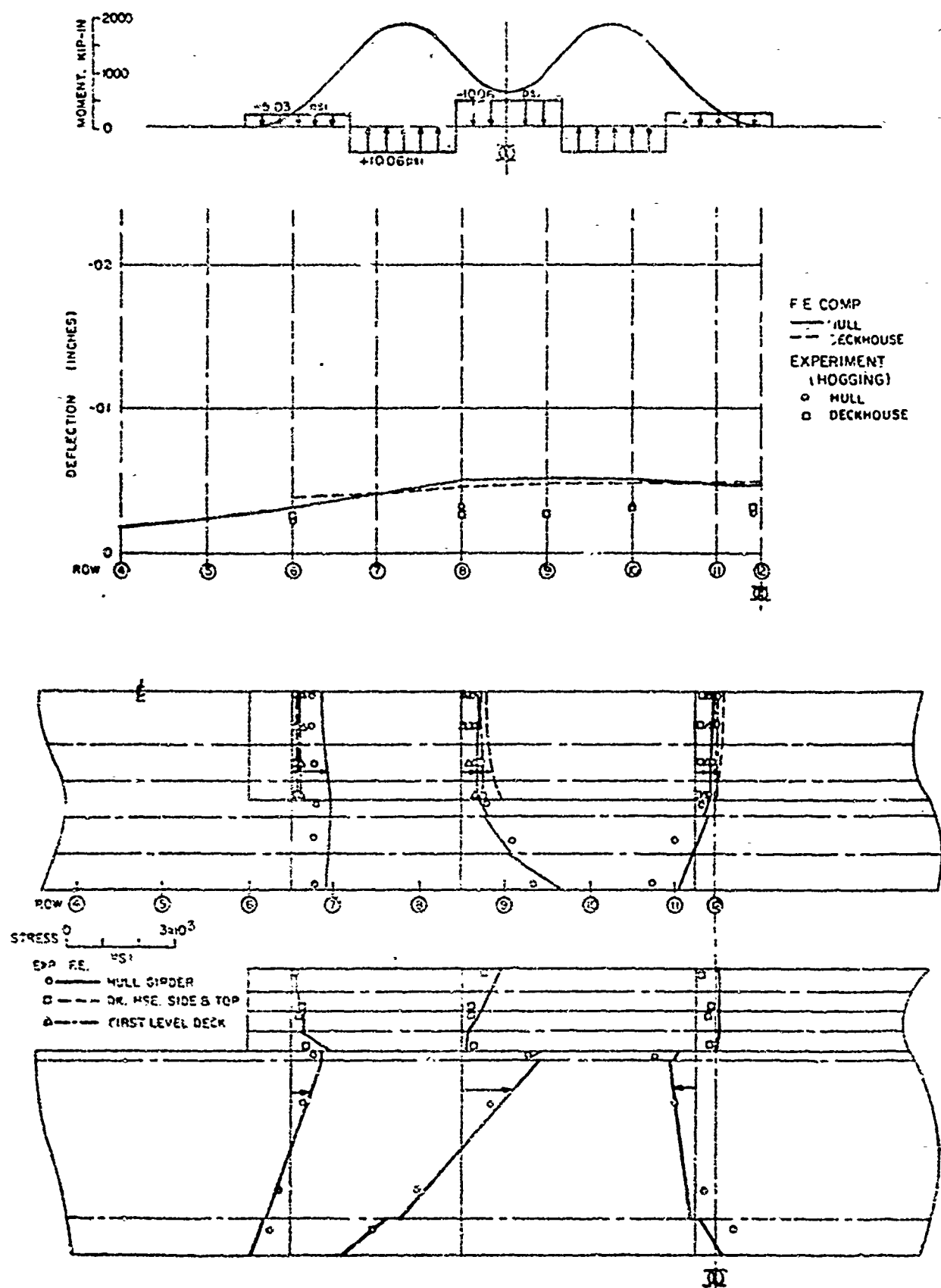


FIGURE 16. DEFLECTIONS AND LONGITUDINAL STRESSES, TEST 9CH



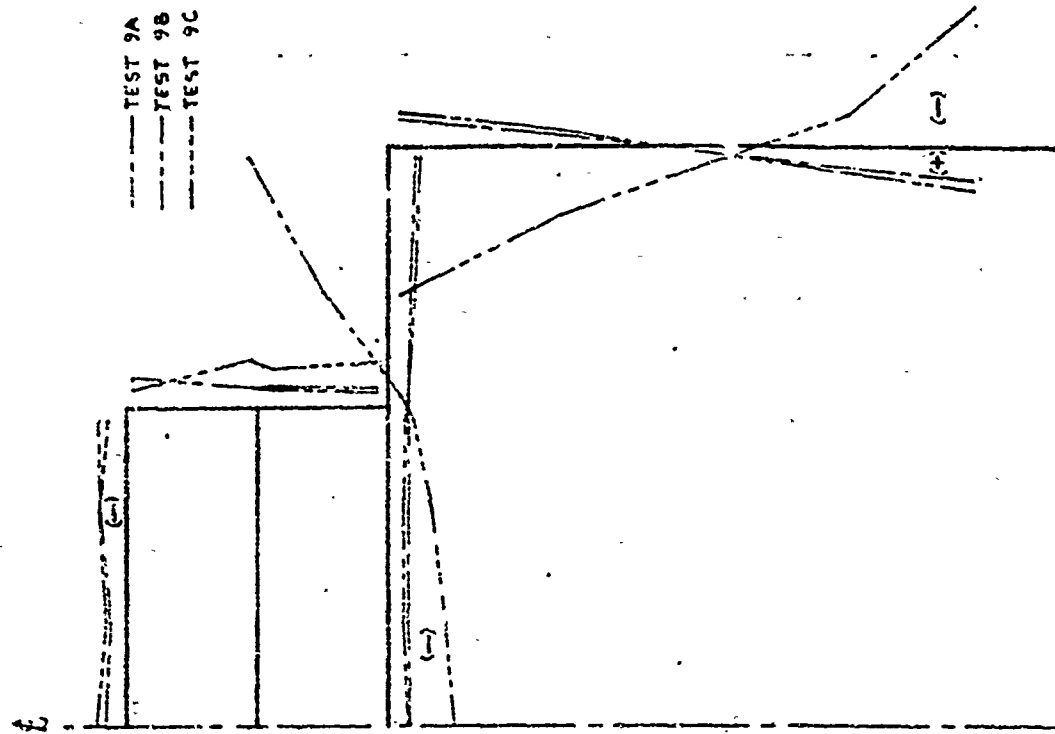


FIGURE 17. STANCHIONS LOOSE

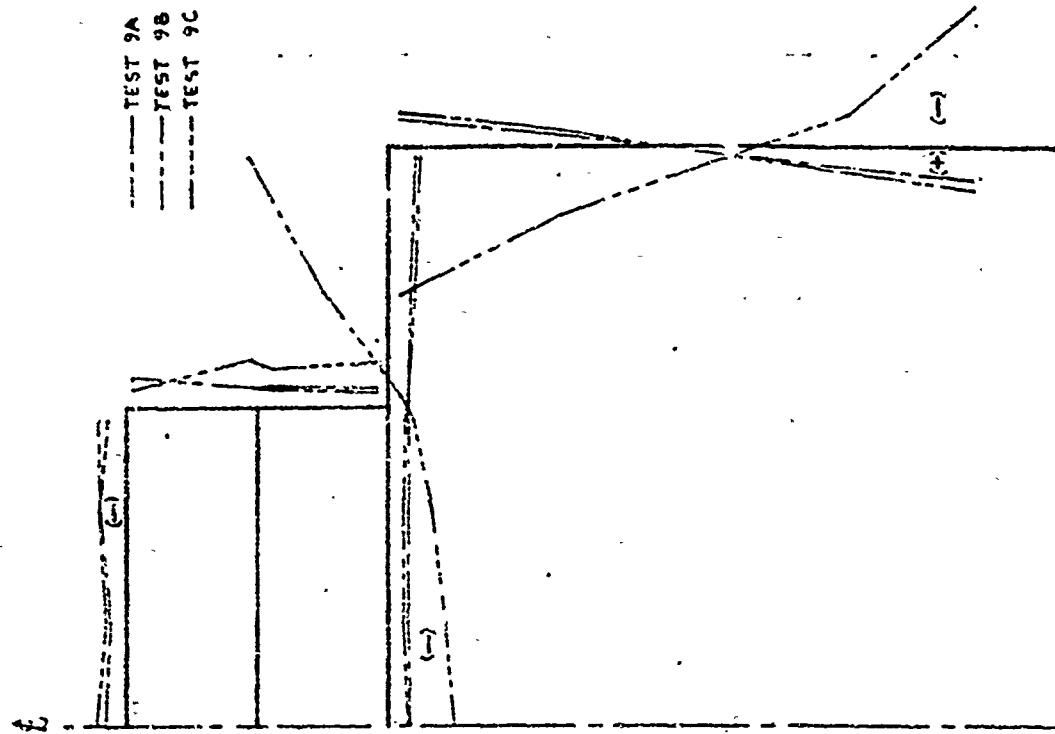


FIGURE 18. STANCHIONS FIXED

### LONGITUDINAL STRESS DISTRIBUTION AMIDSHIPS

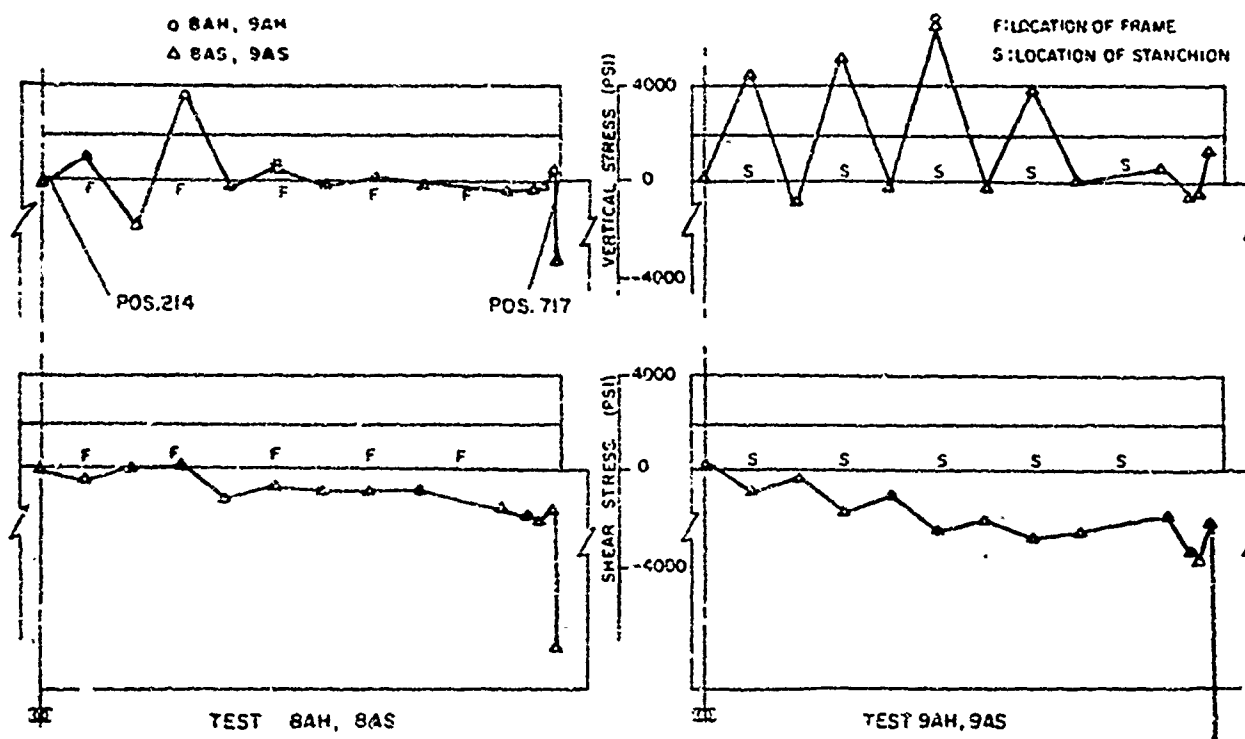


FIGURE 1. SHEAR AND VERTICAL STRESSES AT BOND, TESTS 8A & 9A

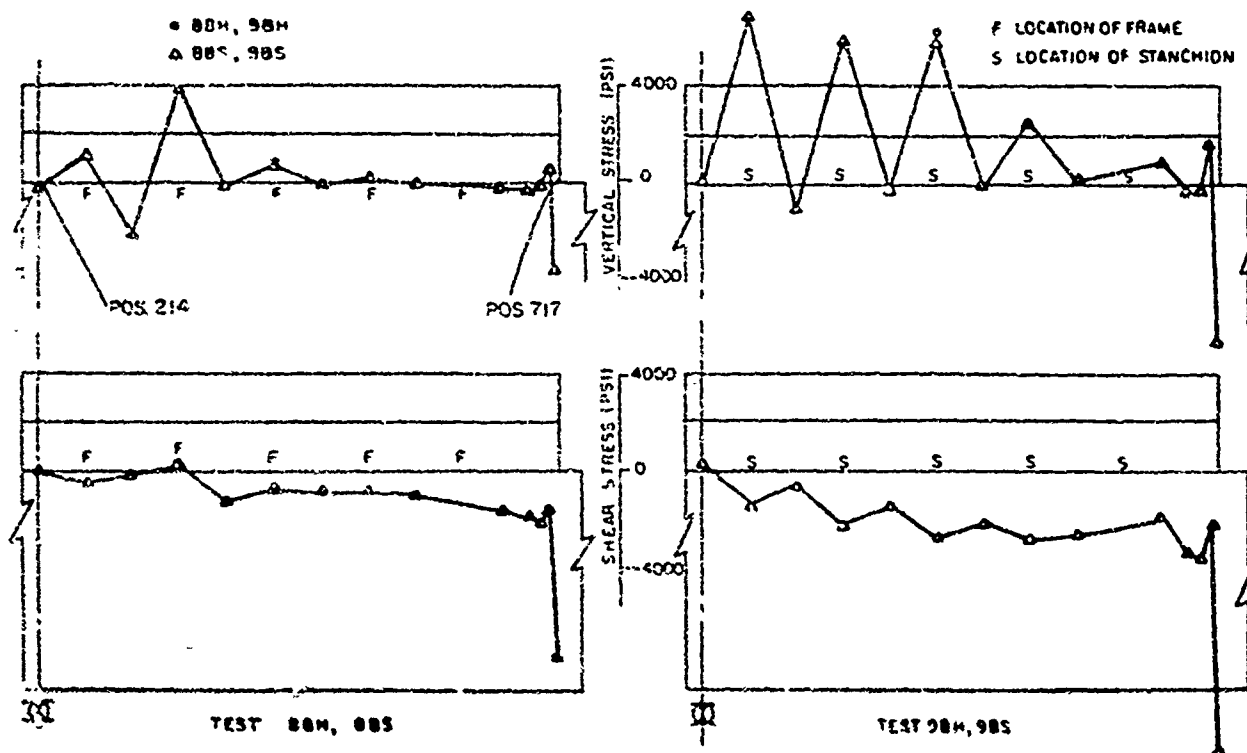


FIGURE 20. SHEAR AND VERTICAL STRESSES AT BOND, TESTS 8B & 9B

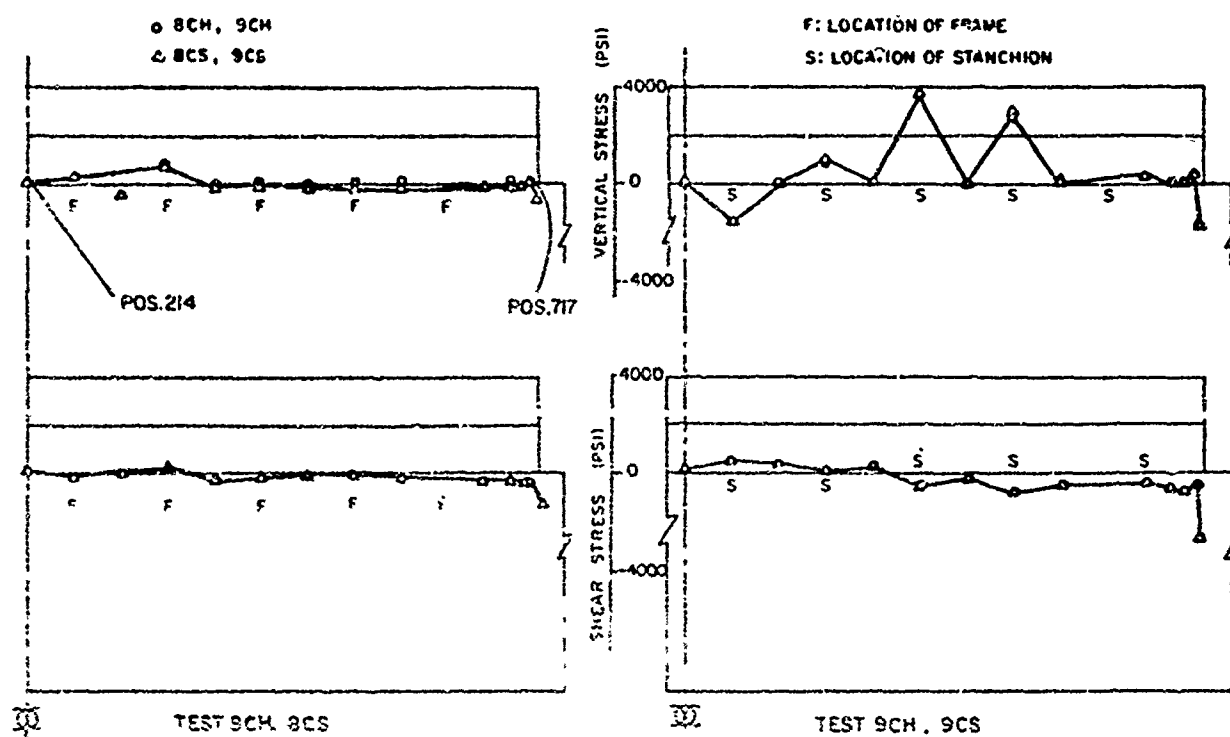


FIGURE 21. SHEAR AND VERTICAL STRESSES AT BOND, TESTS 8C & 9C

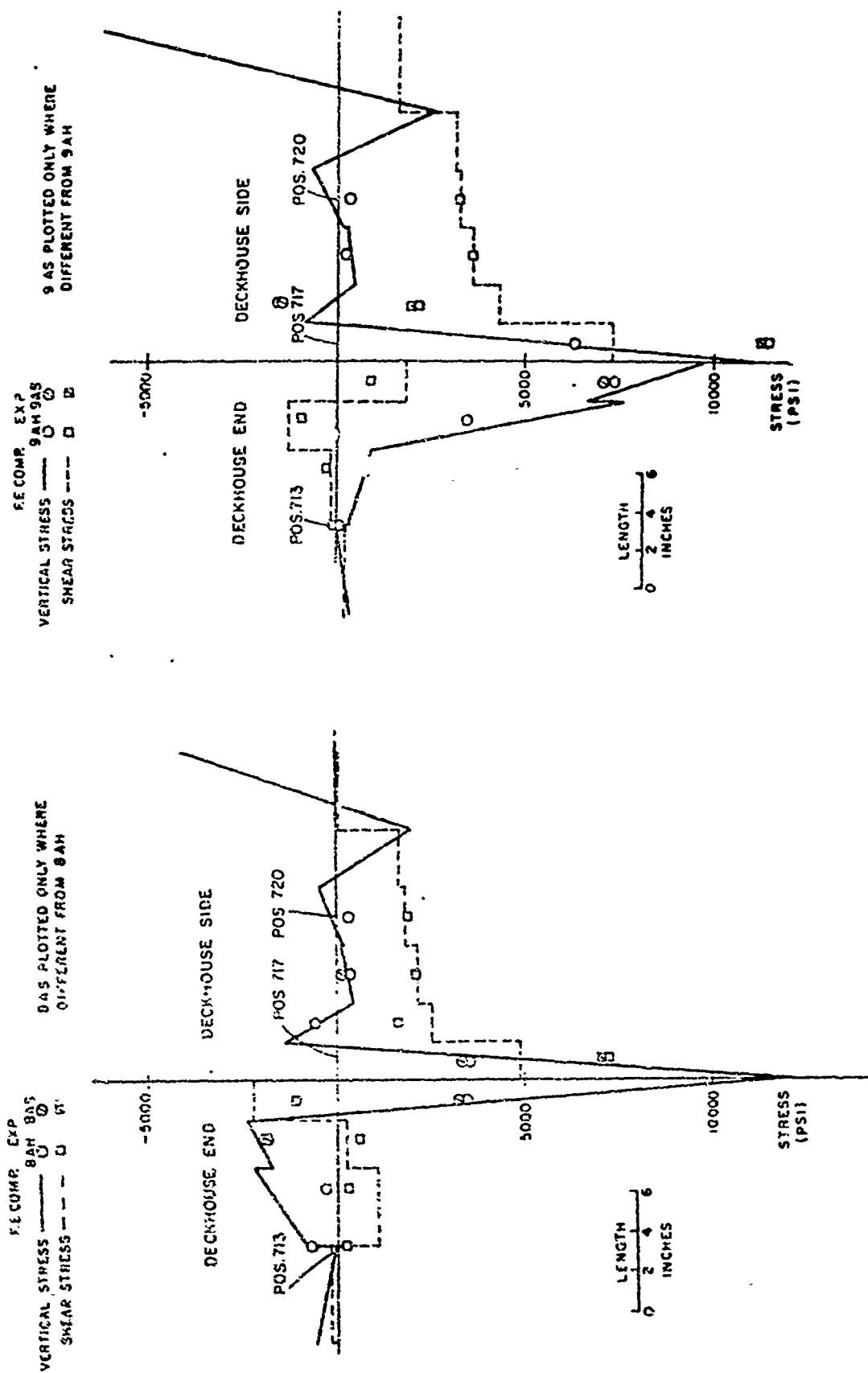


FIGURE 22. TESTS 8AH &amp; 8AS

FIGURE 23. TESTS 9AH &amp; 9AS

SHEAR AND VERTICAL STRESSES AT DECKHOUSE END

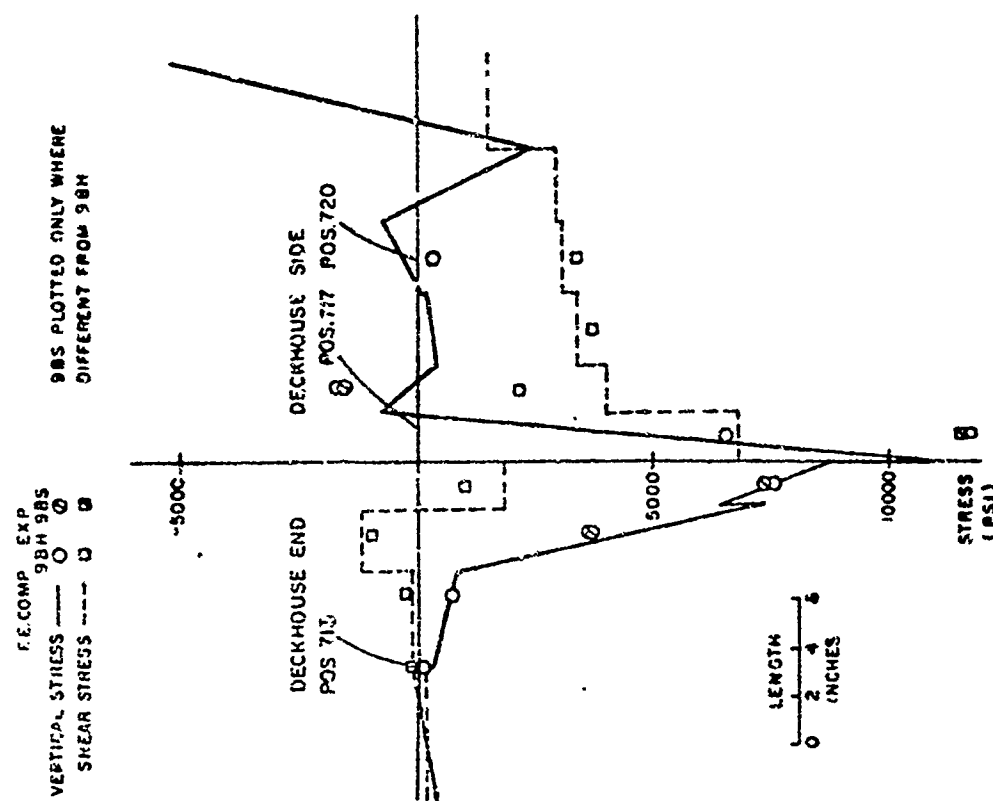
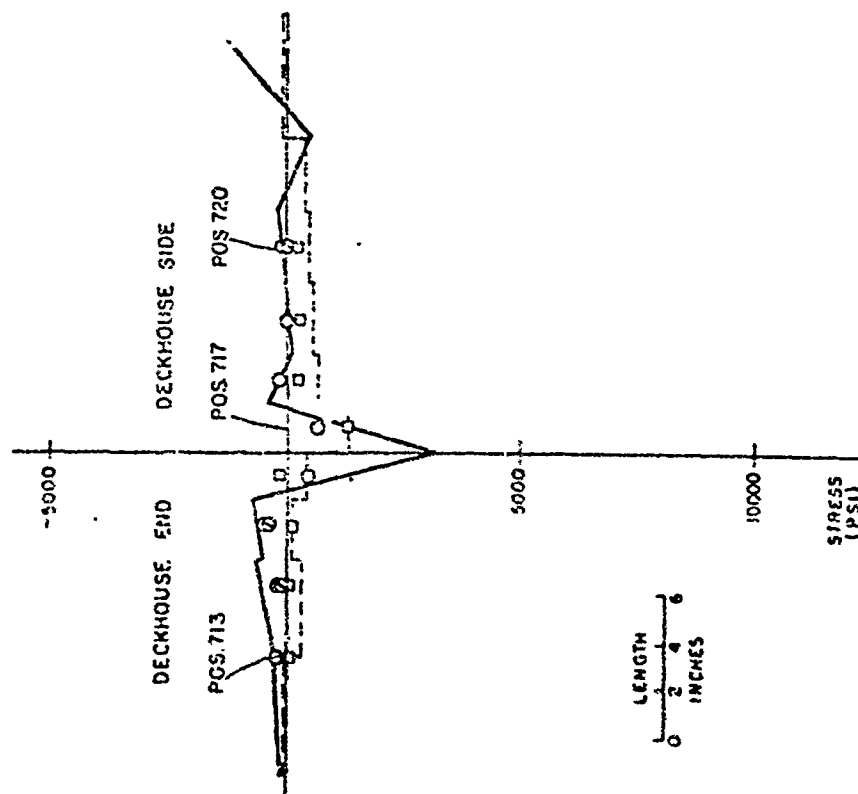


FIGURE 25. TESTS 9BH &amp; 9BS

SHEAR AND VERTICAL STRESSES AT DECKHOUSE END



**FIGURE 26. TESTS 8CH & 8CS**

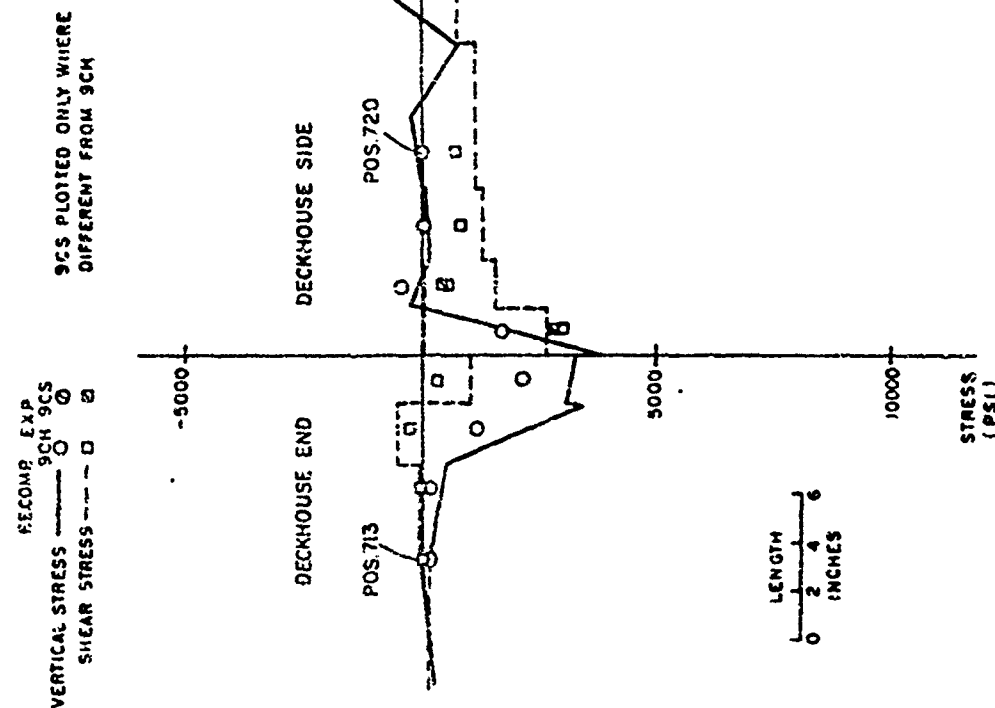


FIGURE 27. TESTS 9CH & 9CS

# SHEAR AND VERTICAL STRESSES AT DECKHOUSE END

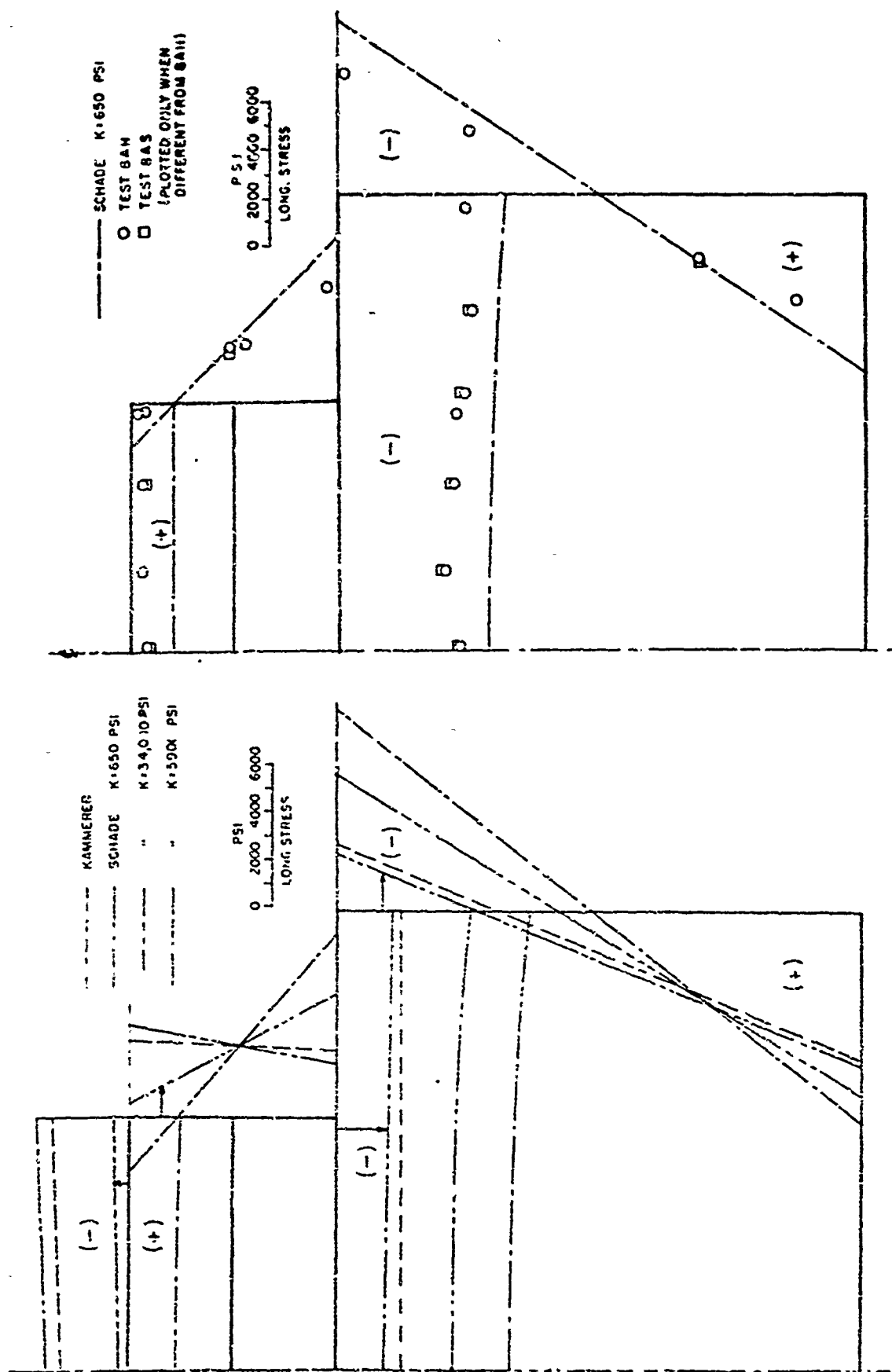


FIGURE 28. COMPARISON OF KAMMERER'S AND SCHADE'S ANALYSIS FOR A TWO-LEVEL DECKHOUSE UNDER BENDING MOMENT B

FIGURE 29. COMPARISON OF TESTS BAH-S AND THEORY AT MIDSHIPS

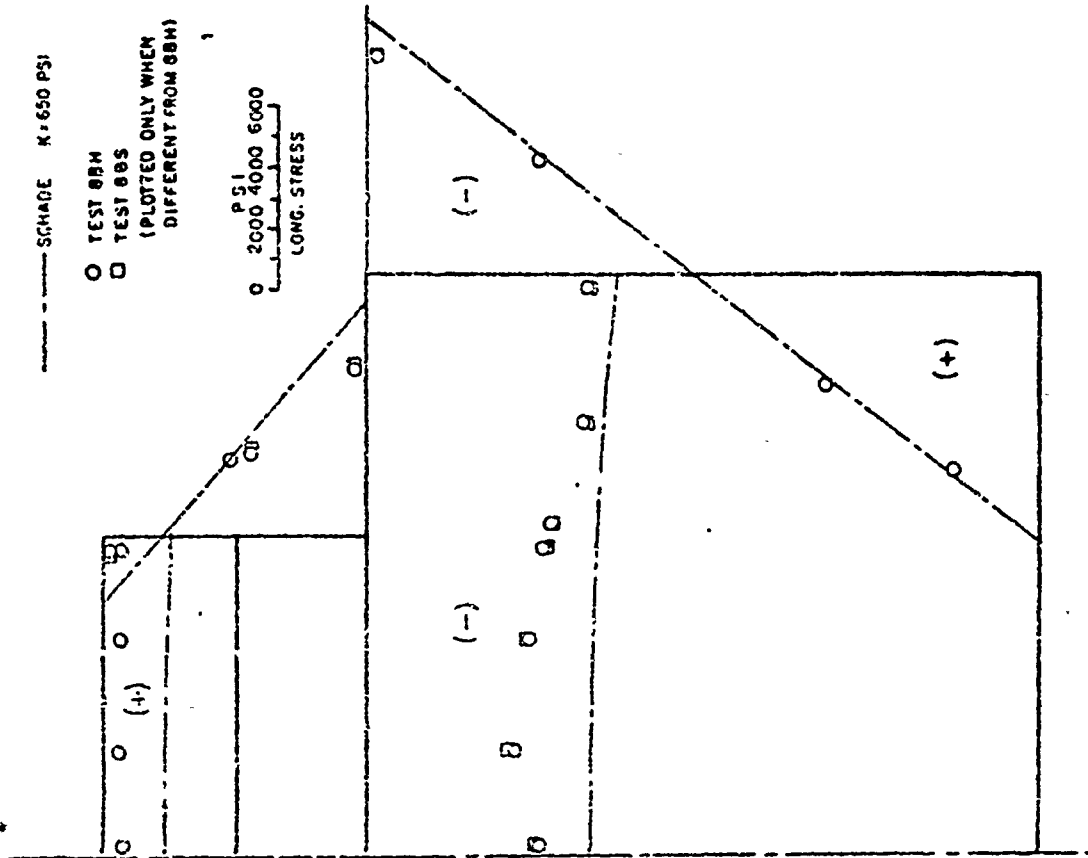


FIGURE 31. COMPARISON OF TESTS 8BH-S AND THEORY AT MIDSHIPS

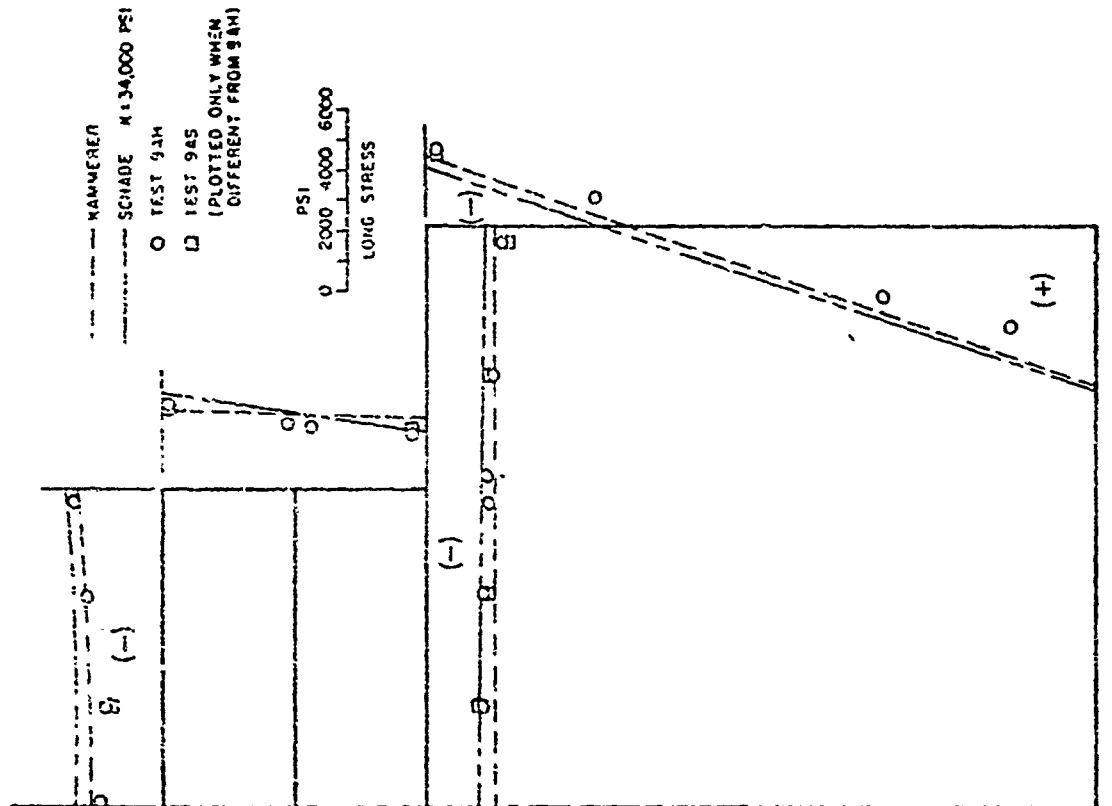


FIGURE 30. COMPARISON OF TESTS 9AH-S AND THEORY AT MIDSHIPS



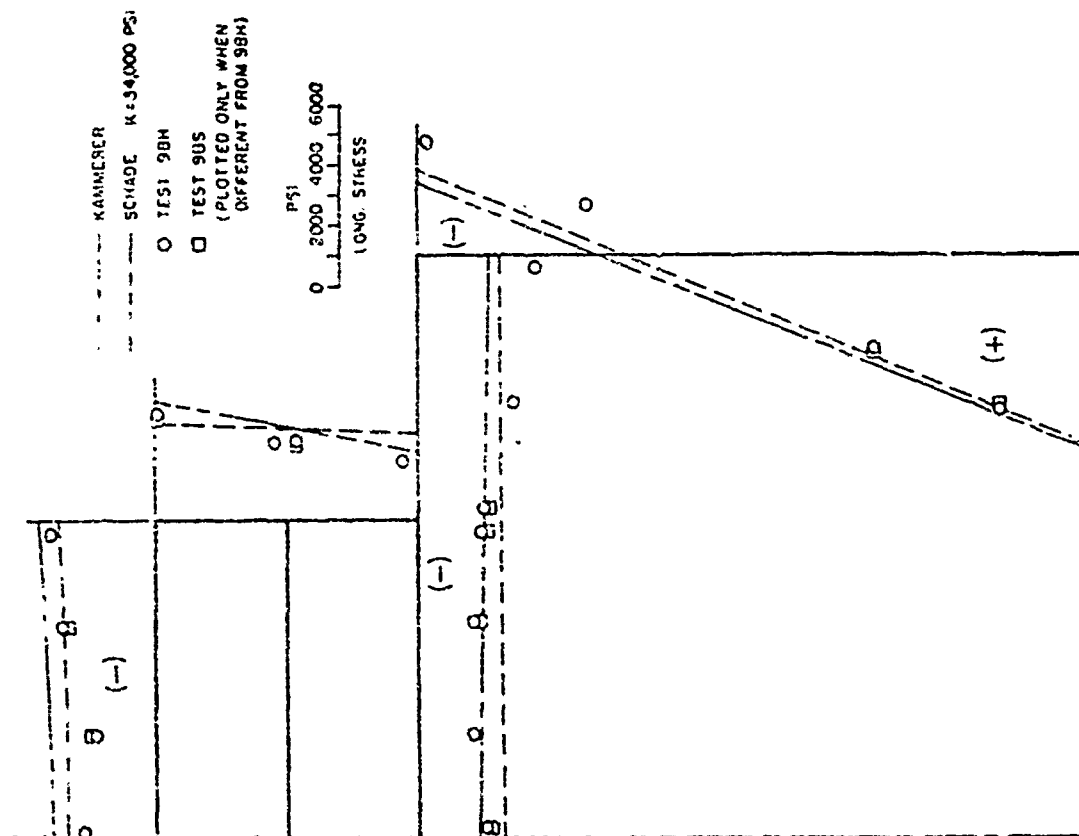


FIGURE 32. COMPARISON OF TESTS 9BH-S AND THEORY AT MIDSHIPS

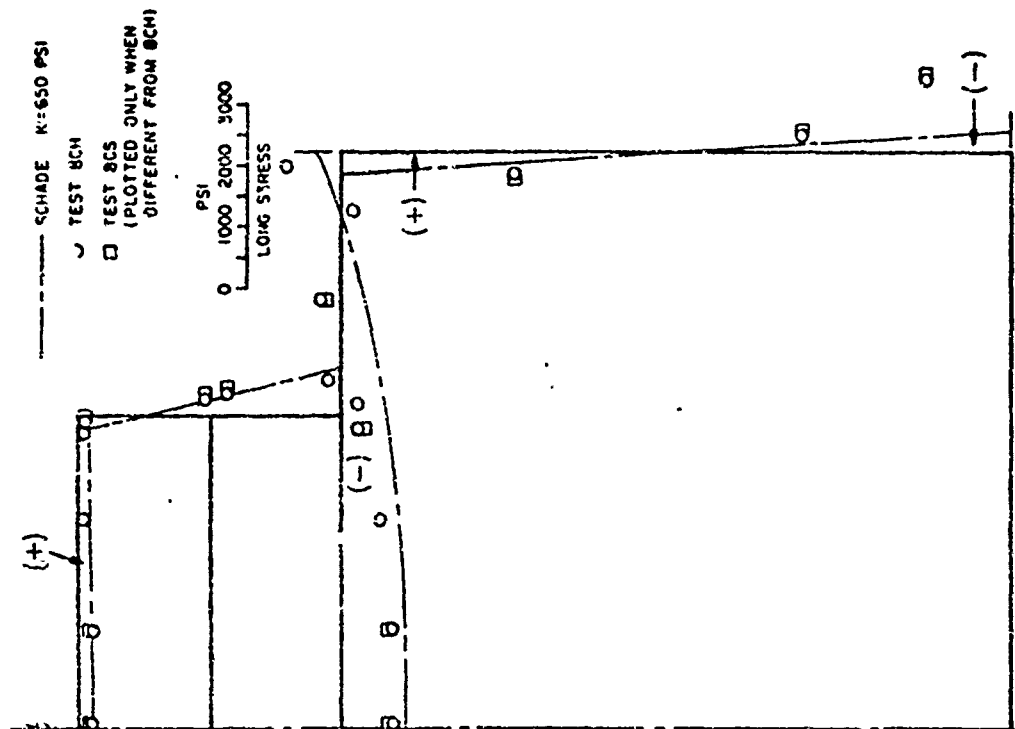


FIGURE 33. COMPARISON OF TESTS 8CH-S AND THEORY AT MIDSHIPS

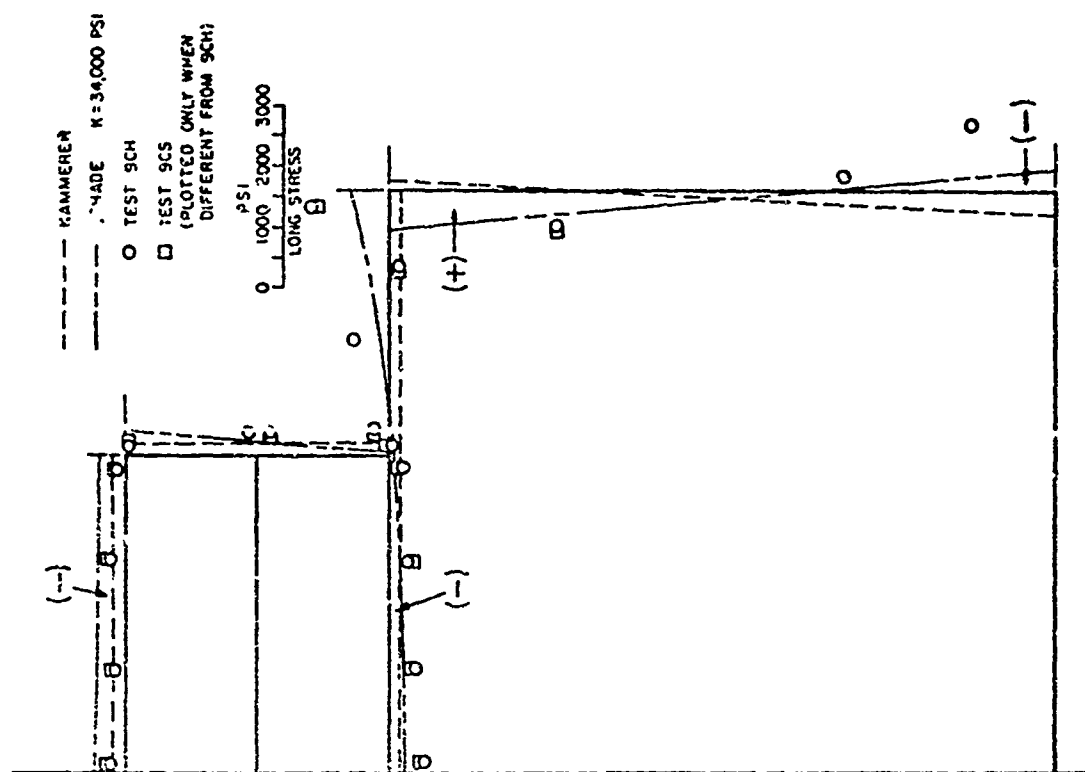


FIGURE 34. COMPARISON OF TESTS  
 9CH-S AND THEORY AT  
 MIDSH..PS

## ACKNOWLEDGEMENTS

The authors are obliged to Professor J. R. Paulling for the guidance and help given throughout the performance of the project. The following students were actively involved in the performance of the tests and the evaluation of the results: Kwang June Bai, Rong Tsao Huang, and Theodore R. Wise. Their assistance is gratefully acknowledged.

The work was made possible by the financial support of the Naval Ship Engineering Center and the Naval Ship Research and Development Center.

## REFERENCES

1. Buhler, Johannes, "Response to Static Loads of a Longitudinal Framed Box Girder Model, Part I", College of Engineering, University of California, Berkeley, Report NA 69-2, May, 1969.
2. Glasfeld, R. D., "Design and Construction of a 42 ft. Structural Model Testing Facility", University of California, IER Report Series 184, Issue 1, Feb., 1962.
3. Glasfeld, R. D., "Preliminaries to Deckhouse Tests on the 42 ft. Structural Model Testing Facility", University of California, Berkeley, Report No. NA 63-4, June 1963.
4. Glasfeld, R. D. & Jan, H. Y., "Structural Model Studies of Hull-Deckhouse Interaction - Part I", University of California, Berkeley, Report No. NA-66-10, Nov., 1966.
5. Jan, Hsien-Yun, "Response to Line Shear Loads of a Box Girder with Orthotropic Envelope Plating", University of California, Berkeley, Report No. NA-67-3, April, 1967.
6. Kammerer, J. T., "A Design Procedure for Determining the Contribution of Deckhouses to the Longitudinal Strength of Ships", Assoc. of Senior Engineers, Bureau of Ships, Third Annual Technical Symposium, March, 1966.
7. Paulling, J. R. & Payer, Hans G., "Hull-Deckhouse Interaction by Finite-Element Calculations", SNAME Transactions, 1968, pp. 281-308.
8. Schade, H. A., "The Effective Breadth Concept in Ship-Structure Design", SNAME Transactions, 1953, pp. 410-430.
9. Schade, H. A., "Thin-Walled Box Girder - Theory and Experiment", University of California, Berkeley, Report No. NA-64-6, June, 1964.
10. Schade, H. A., "Two-Beam Deckhouse Theory with Shear Effects", University of California, Berkeley, Report No. NA-65-3. October, 1965.

## APPENDIX A

TABLE 3

DEFLECTION DATA, in  $\times 10^{-3}$ 

| GAGE | TEST |     |     |     |     |     |     |     |     |     |     |     |
|------|------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|      | 8AH  | 8AS | 8BH | 8BS | 8CH | 8CS | 9AH | 9AS | 9BH | 9BS | 9CH | 9CS |
| A    | 11   | 12  | 10  | 13  | 1   | 1   | 8   | 10  | 8   | 1   | 1   | 1   |
| B    | 147  | 150 | 154 | 157 | 23  | 22  | 126 | 126 | 129 | 129 | 19  | 18  |
| C    | 201  | 201 | 208 | 211 | 40  | 38  | 166 | 166 | 171 | 170 | 32  | 31  |
| D    | 224  | 225 | 244 | 246 | 39  | 36  | 184 | 182 | 196 | 196 | 30  | 30  |
| E    | 226  | 226 | 249 | 251 | 35  | 32  | 186 | 185 | 203 | 202 | 27  | 26  |
| F    | 149  | 149 | 156 | 156 | 24  | 23  | 130 | 128 | 134 | 132 | 20  | 19  |
| G    | 11   | 11  | 12  | 11  | 1   | 1   | 10  | 9   | 10  | 9   | 1   | 1   |
| H    | 9    | 9   | 9   | 9   | 1   | 1   | 7   | 6   | 7   | 7   | 1   | 1   |
| I    | 227  | 227 | -   | 251 | 35  | 32  | 185 | 183 | 201 | 199 | 27  | 26  |
| J    | 147  | 146 | 153 | 153 | 24  | 22  | 124 | 123 | 127 | 126 | 18  | 18  |
| K    | 11   | 12  | 12  | 12  | 2   | 2   | 9   | 9   | 9   | 10  | 1   | 1   |
| 1    | 204  | 203 | 220 | 218 | 34  | 33  | 146 | 146 | 145 | 150 | 24  | 23  |
| 2    | 171  | 173 | 185 | 183 | 30  | 28  | 157 | 157 | 165 | 161 | 27  | 26  |
| 3    | 160  | 160 | 173 | 171 | 28  | 26  | 171 | 170 | 182 | 180 | 29  | 28  |
| 4    | 162  | 162 | 174 | 174 | 28  | 26  | 177 | 175 | 189 | 187 | 29  | 27  |
| 5    | 159  | 162 | 72  | 173 | 27  | 26  | 174 | -   | 187 | -   | 30  | 28  |
| 6    | 203  | 203 | 217 | 217 | 37  | 35  | 142 | 142 | 146 | 145 | 23  | 22  |

The sign of the deflections is shown reversed for sagging.

TABLE 4

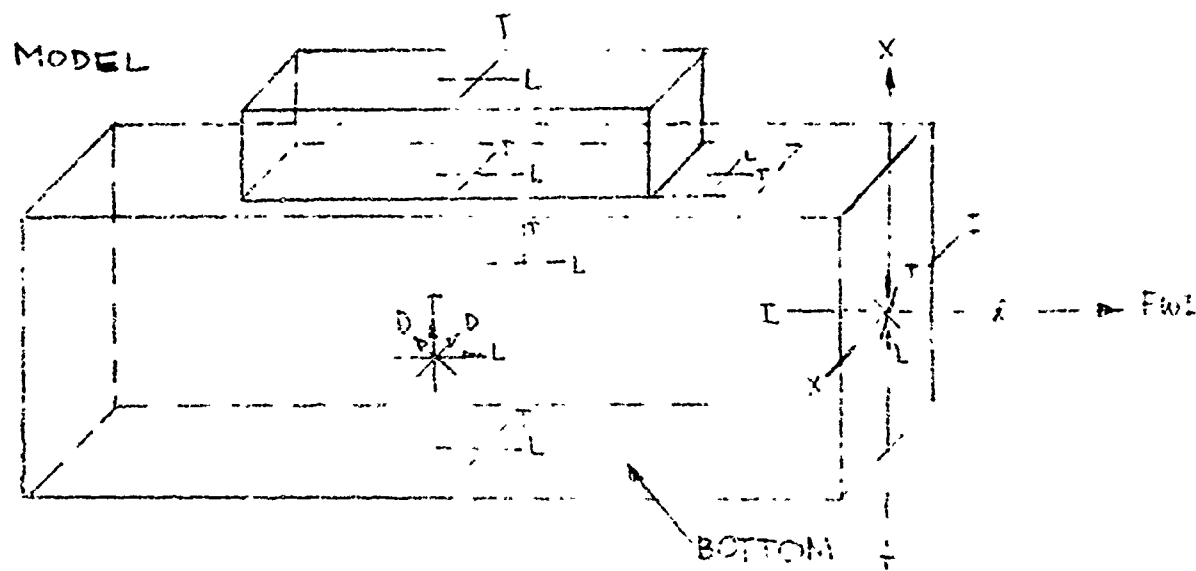
CORRECTED DEFLECTION DATA, in  $\times 10^{-3}$ 

| GAGE            | TEST |     |     |     |     |     |     |     |     |     |     |     |
|-----------------|------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|                 | 8AH  | 8AS | 8BH | 8BS | 8CH | 8CS | 9AH | 9AS | 9BH | 9BS | 9CH | 9CS |
| A               | 31   | 32  | 25  | 28  | 3   | 3   | 23  | 25  | 23  | 26  | 2   | 2   |
| B               | 167  | 170 | 169 | 172 | 25  | 24  | 141 | 141 | 144 | 144 | 20  | 19  |
| C               | 221  | 221 | 223 | 226 | 42  | 40  | 181 | 181 | 186 | 185 | 33  | 32  |
| D               | 244  | 245 | 259 | 261 | 41  | 38  | 199 | 197 | 211 | 211 | 31  | 31  |
| E               | 246  | 246 | 264 | 266 | 37  | 34  | 201 | 200 | 218 | 217 | 28  | 27  |
| F               | 169  | 169 | 171 | 171 | 26  | 25  | 145 | 143 | 149 | 147 | 21  | 20  |
| G               | 31   | 31  | 27  | 26  | 3   | 3   | 25  | 24  | 25  | 24  | 2   | 2   |
| 1               | 224  | 223 | 235 | 233 | 36  | 35  | 161 | 161 | 160 | 165 | 25  | 24  |
| 2               | 191  | 193 | 200 | 198 | 32  | 30  | 172 | 172 | 180 | 179 | 28  | 27  |
| 3               | 180  | 180 | 188 | 186 | 30  | 28  | 186 | 185 | 197 | 195 | 30  | 29  |
| 4               | 182  | 182 | 189 | 189 | 30  | 28  | 192 | 190 | 204 | 202 | 30  | 28  |
| CORREC-<br>TION | +20  | +20 | +15 | +15 | +2  | +2  | +15 | +15 | +15 | +15 | +1  | +1  |

The sign of the deflections is shown reversed for sagging.

APPENDIX B

CODING AS USED IN THE COMPUTER PROGRAMS  
AND IN TABLES 5 THROUGH 10

Location of Gage

X: Top, Starboard or Forward Side.

I: Bottom, Port or Aft Side.

Direction of Gage

L: Longitudinal

T: Transversal

D: Diagonal

Abbreviations

STRS: Stress

STR: Strain

D: Direct

B: Bending

Conventions

Stresses are given in units of psi, strains in  $\mu$  in/in.

Tensile stresses appear with negative sign.

Gages located in marked area have different convention as shown.





[illegible]

```

72=713+714+715+716+717+718+719+720+721+722+723+741+742+751+752
8739+753+755+K
201 A1=X150+J1
A2=X100+J1
A4=X1030+J1
A5=X1039+J1
GO TO 2507
202 A1=X1204+J1
A2=X1940+J1
A4=X1040+J1
A4=X1041+J1
GO TO 2507
203 A1=X1004+J1
A2=X1007+J1
A4=X1042+J1
A5=X1-45+J1
GO TO 2502
204 A1=X18+J1
A2=X19+J1
A4=X144+J1
A4=X145+J1
GO TO 2502
714 A1=X1158+J1
A2=X1159+J1
A3=X116+J1
A4=X1194+J1
A5=X1153+J1
A6=X1196+J1
GO TO 2501
215 A1=X1161+J1
A2=X1162+J1
A4=X1297+J1
A5=X1298+J1
GO TO 2502
+
+
+
+
+
+
741 A1=X122+J1
A4=X1523+J1
GO TO 2505
742 A1=X1324+J1
A4=X1523+J1
GO TO 2505
2501 D1STRS=CONST40+1,50(A1+A4)0,50POISRT0(A2+A5)1
BLSTRS=CONST40+1,50(A1-A4)0,50POISRT0(A2-A5)1
DTSTRS=CONST40+1,50(A2+A5)0,50POISRT0(A1+A4)1
B5STRS=CONST40+1,50(A2-A5)0,50POISRT0(A1-A4)1
P1STRS=CONST40+1,50(A1+A4)0,50(A2+A5)-(A3-A4)1
PR:NT 2075+214+DLSTRS+PLSTRS+PTSTRS+RTSTRS+SmSTRS+Z(K)+A1+A2+A3+A4
1+A5+A6
GO TO 2600
2502 DLSTRS=CONST40+1,50(A1+A4)-,50POISRT0(A2+A5)1
BLSTRS=CONST40+1,50(A1-A4)-,50POISRT0(A2-A5)1
DTSTRS=CONST40+1,50(A2+A5)0,50POISRT0(A1+A4)1
RTSTRS=CONST40+1,50(A2-A5)0,50POISRT0(A1-A4)1
PR:NT 2085+212+DLSTRS+PLSTRS+PTSTRS+RTSTRS+Z(K)+A1+A2+A3+A4
GO TO 2600
2503 DLSTRS=CONST40+1,50(A1+A4)1/2,
BLSTRS=CONST40+1,50(A1-A4)1/2,
PR:NT 2205+214+DLSTRS+PLSTRS+Z(K)+A1+A4
GO TO 2600
2600 CONTINUE
2075 FCMAF 11X,F5,0+5F10,0+13+0F5,0+7F,0+3F8,0
2085 FCMAF 11X,F5,0+4F10,0+23X,F5,0+7F,0+68X,2F8,0
2205 FCMAF 11X,F5,0+2F10,0+43+0F5,0+7F,0+16X+0F8,0
F50

```

## APPENDIX C

### TABLE 5. STRESSES AND STRAINS

[illegible]











**SRG TEST**

[illegible]





### TABLE 9. STRESSES AND STRAINS

[illegible]

|     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |      |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|------|
| 314 | 315 | 316 | 317 | 318 | 319 | 320 | 321 | 322 | 323 | 324 | 325 | 326 | 327 | 328 | 329 | 330 | 331 | 332 | 333 | 334 | 335 | 336 | 337 | 338 | 339 | 340 | 341 | 342 | 343 | 344 | 345 | 346 | 347 | 348 | 349 | 350 | 351 | 352 | 353 | 354 | 355 | 356 | 357 | 358 | 359 | 360 | 361 | 362 | 363 | 364 | 365 | 366 | 367 | 368 | 369 | 370 | 371 | 372 | 373 | 374 | 375 | 376 | 377 | 378 | 379 | 380 | 381 | 382 | 383 | 384 | 385 | 386 | 387 | 388 | 389 | 390 | 391 | 392 | 393 | 394 | 395 | 396 | 397 | 398 | 399 | 400 | 401 | 402 | 403 | 404 | 405 | 406 | 407 | 408 | 409 | 410 | 411 | 412 | 413 | 414 | 415 | 416 | 417 | 418 | 419 | 420 | 421 | 422 | 423 | 424 | 425 | 426 | 427 | 428 | 429 | 430 | 431 | 432 | 433 | 434 | 435 | 436 | 437 | 438 | 439 | 440 | 441 | 442 | 443 | 444 | 445 | 446 | 447 | 448 | 449 | 450 | 451 | 452 | 453 | 454 | 455 | 456 | 457 | 458 | 459 | 460 | 461 | 462 | 463 | 464 | 465 | 466 | 467 | 468 | 469 | 470 | 471 | 472 | 473 | 474 | 475 | 476 | 477 | 478 | 479 | 480 | 481 | 482 | 483 | 484 | 485 | 486 | 487 | 488 | 489 | 490 | 491 | 492 | 493 | 494 | 495 | 496 | 497 | 498 | 499 | 500 | 501 | 502 | 503 | 504 | 505 | 506 | 507 | 508 | 509 | 510 | 511 | 512 | 513 | 514 | 515 | 516 | 517 | 518 | 519 | 520 | 521 | 522 | 523 | 524 | 525 | 526 | 527 | 528 | 529 | 530 | 531 | 532 | 533 | 534 | 535 | 536 | 537 | 538 | 539 | 540 | 541 | 542 | 543 | 544 | 545 | 546 | 547 | 548 | 549 | 550 | 551 | 552 | 553 | 554 | 555 | 556 | 557 | 558 | 559 | 560 | 561 | 562 | 563 | 564 | 565 | 566 | 567 | 568 | 569 | 570 | 571 | 572 | 573 | 574 | 575 | 576 | 577 | 578 | 579 | 580 | 581 | 582 | 583 | 584 | 585 | 586 | 587 | 588 | 589 | 590 | 591 | 592 | 593 | 594 | 595 | 596 | 597 | 598 | 599 | 600 | 601 | 602 | 603 | 604 | 605 | 606 | 607 | 608 | 609 | 610 | 611 | 612 | 613 | 614 | 615 | 616 | 617 | 618 | 619 | 620 | 621 | 622 | 623 | 624 | 625 | 626 | 627 | 628 | 629 | 630 | 631 | 632 | 633 | 634 | 635 | 636 | 637 | 638 | 639 | 640 | 641 | 642 | 643 | 644 | 645 | 646 | 647 | 648 | 649 | 650 | 651 | 652 | 653 | 654 | 655 | 656 | 657 | 658 | 659 | 660 | 661 | 662 | 663 | 664 | 665 | 666 | 667 | 668 | 669 | 670 | 671 | 672 | 673 | 674 | 675 | 676 | 677 | 678 | 679 | 680 | 681 | 682 | 683 | 684 | 685 | 686 | 687 | 688 | 689 | 690 | 691 | 692 | 693 | 694 | 695 | 696 | 697 | 698 | 699 | 700 | 701 | 702 | 703 | 704 | 705 | 706 | 707 | 708 | 709 | 710 | 711 | 712 | 713 | 714 | 715 | 716 | 717 | 718 | 719 | 720 | 721 | 722 | 723 | 724 | 725 | 726 | 727 | 728 | 729 | 730 | 731 | 732 | 733 | 734 | 735 | 736 | 737 | 738 | 739 | 740 | 741 | 742 | 743 | 744 | 745 | 746 | 747 | 748 | 749 | 750 | 751 | 752 | 753 | 754 | 755 | 756 | 757 | 758 | 759 | 760 | 761 | 762 | 763 | 764 | 765 | 766 | 767 | 768 | 769 | 770 | 771 | 772 | 773 | 774 | 775 | 776 | 777 | 778 | 779 | 780 | 781 | 782 | 783 | 784 | 785 | 786 | 787 | 788 | 789 | 790 | 791 | 792 | 793 | 794 | 795 | 796 | 797 | 798 | 799 | 800 | 801 | 802 | 803 | 804 | 805 | 806 | 807 | 808 | 809 | 810 | 811 | 812 | 813 | 814 | 815 | 816 | 817 | 818 | 819 | 820 | 821 | 822 | 823 | 824 | 825 | 826 | 827 | 828 | 829 | 830 | 831 | 832 | 833 | 834 | 835 | 836 | 837 | 838 | 839 | 840 | 841 | 842 | 843 | 844 | 845 | 846 | 847 | 848 | 849 | 850 | 851 | 852 | 853 | 854 | 855 | 856 | 857 | 858 | 859 | 860 | 861 | 862 | 863 | 864 | 865 | 866 | 867 | 868 | 869 | 870 | 871 | 872 | 873 | 874 | 875 | 876 | 877 | 878 | 879 | 880 | 881 | 882 | 883 | 884 | 885 | 886 | 887 | 888 | 889 | 890 | 891 | 892 | 893 | 894 | 895 | 896 | 897 | 898 | 899 | 900 | 901 | 902 | 903 | 904 | 905 | 906 | 907 | 908 | 909 | 910 | 911 | 912 | 913 | 914 | 915 | 916 | 917 | 918 | 919 | 920 | 921 | 922 | 923 | 924 | 925 | 926 | 927 | 928 | 929 | 930 | 931 | 932 | 933 | 934 | 935 | 936 | 937 | 938 | 939 | 940 | 941 | 942 | 943 | 944 | 945 | 946 | 947 | 948 | 949 | 950 | 951 | 952 | 953 | 954 | 955 | 956 | 957 | 958 | 959 | 960 | 961 | 962 | 963 | 964 | 965 | 966 | 967 | 968 | 969 | 970 | 971 | 972 | 973 | 974 | 975 | 976 | 977 | 978 | 979 | 980 | 981 | 982 | 983 | 984 | 985 | 986 | 987 | 988 | 989 | 990 | 991 | 992 | 993 | 994 | 995 | 996 | 997 | 998 | 999 | 1000 |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|------|





## APPENDIX D

A. Coefficients for Expansion of B.M. over Length of Hull

| B.M. | Fourier Coef. icients (Sine Series) KIP-IN |                |                |                |                |                 |                 |                 |
|------|--|----------------|----------------|----------------|----------------|-----------------|-----------------|-----------------|
|      | a <sub>1</sub>                             | a <sub>3</sub> | a <sub>5</sub> | a <sub>7</sub> | a <sub>9</sub> | a <sub>11</sub> | a <sub>13</sub> | a <sub>15</sub> |
| A    | 7332.9                                     | -949.01        | -533.85        | -0.0342        | -91.857        | -19.473         | 3.4403          | 2.2414          |
| B    | 7808.1                                     | -1385.1        | -249.56        | -141.85        | -42.920        | -28.414         | 3.6958          | 2.4009          |
| C    | 1253.7                                     | -252.29        | -460.84        | 368.70         | -79.319        | -5.1688         | 0.5841          | 0.3867          |

B. Coefficients for Expansion of B.M. over Length of Deckhouse

| B.M. | Fourier Coefficients (Constant + Sine Series ) KIP-IN |                |                |                |                |                |                 |                 |                 |
|------|---|----------------|----------------|----------------|----------------|----------------|-----------------|-----------------|-----------------|
|      | m   | a <sub>1</sub> | a <sub>3</sub> | a <sub>5</sub> | a <sub>7</sub> | a <sub>9</sub> | a <sub>11</sub> | a <sub>13</sub> | a <sub>15</sub> |
| A    | 4482.3  | 3625.5         | 398.64         | -37.444        | -5.4501        | -1.8476        | -12.944         | -1.0843         | -2.2294         |
| B    | 4482.3  | 4580.9         | -37.637        | 26.376         | -5.5713        | -7.4400        | -0.0113         | -4.3621         | -2.2753         |
| C    | 554.95  | 939.12         | 770.88         | -137.18        | -21.705        | -5.8589        | -33.633         | -3.4325         | -8.9022         |

TABLE 11

# APPENDIX E

## Computations for Comparison of Kammerer's and Schade's Methods of Deckhouse Analysis with Experimental Data

### Basic Nomenclature (Kammerer - Ref. 6)

- $A_{DE}$  = equivalent area of deckhouse
- $A_H$  = area of hull
- $B$  = beam of ship
- $C_E$  = distance from the strength deck to the effective  
neutral axis of hull area and equivalent deckhouse  
area
- $C_H$  = distance from the strength deck to hull neutral axis
- $D_H$  = depth of hull
- $I_E$  = effective moment of inertia of hull and deckhouse
- $I_H$  = moment of inertia of hull
- $K_3$  = shear lag factor
- $L$  = length of ship
- $M$  = bending moment generated by external forces
- $M_{DE}$  = statical moment of the equivalent area of the deck-  
house about the effective neutral axis
- $b$  = width of deckhouse
- $h$  = height of deckhouse
- $l$  = length of deckhouse
- $l_E$  = effective length of deckhouse
- $\sigma_D$  = longitudinal stress at the strength deck
- $\sigma_K$  = longitudinal stress at the keel
- $\sigma_T$  = longitudinal stress at top of deckhouse

### Basic Nomenclature (Schade - Ref. 10)

The external loading is applied to the primary member (the hull) identified by the subscript 1; the "following" member (deckhouse or superstructure) is identified by the subscript f.

|                                       |   |
|---------------------------------------|---|
| $\sigma_1$ and $\sigma_f$             | longitudinal stress in sides (x-direction)                            |
| $\bar{\sigma}_1$ and $\bar{\sigma}_f$ | longitudinal stress in sides at deck (x-direction)                    |
| $p_1$ and $p_f$                       | average longitudinal stress (x-direction)                             |
| $M_1$ and $M_f$                       | stress moment   |
| $S_1$ and $S_f$                       | section modulus referred to deck                                      |
| $E_1$ and $E_f$                       | material modulus - normal   |
| $G_1$ and $G_f$                       | material modulus - shear  |
| $A_1$ and $A_f$                       | effective section area including webs and flanges ( $A = A_1 + A_f$ ) |
| $a_1$ and $a_f$                       | section area (webs only)  |
| $I_1$ and $I_f$                       | section moment of inertia   |
| $q_1$ and $q_f$                       | vertical load/unit length ( $q = q_1 + q_f$ )                         |
| $w_1$ and $w_f$                       | vertical deflection (+ down)  |
| $Q_1$ and $Q_f$                       | vertical shear force ( $Q = Q_1 + Q_f$ )                              |
| $e_1$ and $e_f$                       | distance from bond to individual N.A. ( $e = e_1 + e_f$ )             |
| $z_1$ and $z_f$                       | vertical coordinate distance from individual N.A. (+ down)            |
| 2b                                    | beam of hull  |
| y                                     | transverse coordinate distance from centerline                        |
| k                                     | spring constant of hull at bond                                       |
| $\bar{N}$                             | longitudinal shear flow in deckhouse at bond                          |
| (k)                                   | shear deflection factor   |
| M                                     | bending moment generated by external forces                           |
| m                                     | constant component of bending moment                                  |



- a            amplitude of sinusoidal component of bending moment
- $F_x$         longitudinal external force on hull
- $2\lambda$         length of deckhouse
- $\rho$           effectiveness ratio

Parameters Used for Computations

$$B \equiv \frac{A_1 r S_f}{(A_1 + r A_f) S_1 S_f + A_1 A_f (e_f S_1 + e_1 r S_f)}, \text{ in.}^{-3} \quad [E-1]$$

$$C \equiv \frac{A_1 (r e_1 + e_f)}{(A_1 + r A_f) (I_1 + I_f) + A_1 A_f e (r e_1 + e_f)}, \text{ in.}^{-3} \quad [E-2]$$

$$\delta \equiv 1 + \frac{\eta}{\omega^2} \left( \frac{\pi}{2\lambda} \right)^2 + \frac{1}{4\omega^4} \left( \frac{\pi}{2\lambda} \right)^4 \quad [E-3]$$

$$\theta \equiv 1 + \frac{E}{r e_1 + e_f} \left[ \frac{I_1 e_f}{a_1 G} + \frac{r I_f e_1}{a_f G} \right] \left( \frac{\pi}{2\lambda} \right)^2 + \frac{E}{r e_1 + e_f} \frac{r I_f e_1}{k} \left( \frac{\pi}{2\lambda} \right)^4 \quad [E-4]$$

$$\omega \equiv \sqrt{\frac{k}{4r I_f} \frac{(r e_1 + e_f)}{E e_1} \frac{B}{C}}, \text{ in.}^{-1} \quad [E-5]$$

$$\eta = \frac{1}{2G} \left( \frac{1}{a_1} + \frac{1}{a_f} \right) \sqrt{\frac{k E r I_f e_1}{(r e_1 + e_f) \frac{C}{B}}} \quad [E-6]$$

$$J^2 = \frac{\omega^2}{\eta} = \frac{1}{\frac{E}{G} \left( \frac{1}{a_1} + \frac{1}{a_f} \right)} \frac{(A_1 + A_f r) (I_1 + I_f) + A_1 A_f e (r e_1 + e_f)}{(A_1 + A_f r) I_1 I_f + A_1 A_f (I_1 e^2 + I_f e_1^2 r)}, \text{ in.}^{-2} \quad [E-7]$$

$$r = \frac{1}{2} \frac{1}{\cosh \frac{\pi b}{2\lambda}} \left[ \frac{\pi Y}{2\lambda} \sinh \frac{\pi Y}{2\lambda} + 2 \cosh \frac{\pi Y}{2\lambda} - \frac{\pi b}{2\lambda} \tanh \frac{\pi b}{2\lambda} \cosh \frac{\pi Y}{2\lambda} \right] \quad [E-8]$$

$\psi, \phi$     Parameters for numerical solution for  $p_f$ .

Equations

$$M = m + a \sin \frac{\pi x}{2\lambda} \quad [\text{E-9}]$$

$$P_f = -C\psi m + [(B\delta - C\theta)\phi - B \sin \frac{\pi x}{2\lambda}]a \quad [\text{E-10}]$$

$$\bar{\sigma}_1 = - \frac{1}{S_1 - rS_f} \left[ \left( \frac{A_f}{A_1} S_1 + S_f + A_f e \right) P_f + M \right] \quad [\text{E-11}]$$

$$\bar{\sigma}_f = r \bar{\sigma}_1 \quad [\text{E-12}]$$

$$\sigma_1 = \left( 1 + \frac{z_1}{e_1} \right) p_1 - \frac{z_1}{e_1} \bar{\sigma}_1 \quad [\text{E-13}]$$

$$\sigma_f = \left( 1 - \frac{z_f}{e_f} \right) p_f + \frac{z_f}{e_f} \bar{\sigma}_f \quad [\text{E-14}]$$

$$p_1 = - \frac{A_f}{A_1} p_f \quad [\text{E-15}]$$

### DETAILS OF CALCULATION

#### A. Application of Kammerer's Method to the Model

The hull girder of the model was designed to simulate the midship portion of a Mariner and constructed on a 1:9.5 scale. The corresponding full-scale dimensions are:

$$\begin{aligned} L &= 528' \\ B &= 76' \\ D_H &= 44.5' \end{aligned}$$

The geometric properties of the hull girder, considered as a simple beam, are:

$$\begin{aligned} A_H &= 50.35 \text{ in}^2 \\ I_H &= 29,553 \text{ in}^4 \\ C_H &= 29.49 \text{ in } (C_H = 23.35' \text{ in full-scale}) \end{aligned}$$

The corresponding full-scale dimensions of the deckhouse are:

$$\begin{aligned} \lambda &= \lambda_E = 209.15' \\ b &= 41.76' \\ h &= 17.76' \\ \ell/L &= 0.396 \\ b/\lambda &= 0.200 \\ \sigma_T/\sigma_D &= 1.33 \text{ for } C_H = 15' \text{ (Kammerer's Fig. 7)} \\ \sigma_T/\sigma_D &= 1.33 \times 0.842 = 1.13 \text{ for } C_H = 23.35' \\ &\quad \text{(Kammerer's Fig. 8)} \\ K_3 &= 0.943 \text{ (Kammerer's Fig. 10)} \end{aligned}$$

The geometric properties of the deckhouse are:

| Item         | Actual Area<br>[in <sup>2</sup> ] | Factors      | Equiv.<br>Area<br>[in <sup>2</sup> ] | Lever<br>[in] | Moment<br>[in <sup>2</sup> -in] |
|--------------|-----------------------------------|--------------|--------------------------------------|---------------|---------------------------------|
| 2nd Level Dk | 10.41                             | 1.13 x 0.943 | 11.09                                | 22.3          | 247.3                           |
| T-Bars       | 1.58                              | 1.13 x 0.943 | 1.68                                 | 23.0          | 38.6                            |
| Angles       | 1.05                              | 1.13         | 1.19                                 | 21.8          | 25.9                            |
| Side Plating | 4.31                              | 1.09         | 4.70                                 | 16.1          | 75.7                            |
| 1st Level Dk | 9.75                              | 1.06 x 0.943 | 9.75                                 | 11.0          | 107.2                           |
| T-Bars       | 1.58                              | 1.06 x 0.943 | 1.58                                 | 11.7          | 18.5                            |
| Angles       | 1.05                              | 1.06         | 1.11                                 | 10.5          | 11.7                            |
| Side Plating | 3.75                              | 1.03         | 3.86                                 | 5.0           | 19.3                            |

$$A_{DE} = 34.96$$

$$*C_E = 17.4 \quad \underline{608.3}$$

$$M_{DE} = 1152.5$$

$$*C_E = \frac{A_H C_H}{A_H + A_{DE}} = \frac{50.35 \times 29.49}{50.35 + 34.96} = 17.4 \text{ in}$$

$$I_E = I_H + A_H (C_H - C_E)^2 + M_{DE} C_E$$

$$= 29,553 + 50.35(29.5 - 17.4)^2 + 1152.5(17.4)$$

$$I_E = 56,979 \text{ in}^4$$

For bending moment B,  $M = 9.123 \times 10^6$  lbs-in at midship.

Therefore:

$$\sigma_D = \frac{-MC_E}{I_E} = \frac{-9.123 \times 10^6 \text{ lb-in} (17.4 \text{ in})}{56,979 \text{ in}^4} = -2786 \text{ psi}$$

$$\sigma_T = \left( \frac{\sigma_T}{\sigma_D} \right) \sigma_D = 1.13(-2786 \text{ psi}) = -3148 \text{ psi}$$

$$\sigma_K = \frac{M(D_H - C_E)}{I_E} = \frac{9.123 \times 10^6 \text{ lb-in} (56.0 - 17.4) \text{ in}}{56,979 \text{ in}^4} = +6180 \text{ psi.}$$

Above results are plotted in Figure 28.

#### B. Application of Schade's Method to the Model

$$m = 4.482 \times 10^6 \text{ lbs-in, } a = 4.641 \times 10^6 \text{ lbs-in (moment B)}$$

$$k = 6.50 \times 10^2 \text{ psi (all stanchions loose)}$$

$$k = 3.40 \times 10^4 \text{ psi (all stanchions fixed)}$$

#### Effective Geometric Properties:

|          |               | For Constant B.M. | For Sinusoidal B.M. |
|----------|---------------|-------------------|---------------------|
| r        | [Eq.E-8]      | 1.00 [Ref.5]      | 0.81                |
| $\rho_1$ | [Ref. 8]      | 1.00              | 0.82                |
| $a_1$    | $\text{in}^2$ | 17.56             | 17.56               |
| $A_1$    | $\text{in}^2$ | 50.35             | 44.45               |
| $e_1$    | in            | 29.49             | 29.39               |
| $I_1$    | $\text{in}^4$ | 29,553            | 25,008              |
| $S_1$    | $\text{in}^3$ | 1002.1            | 850.9               |
| $\rho_f$ | [Ref. 8]      | 0.93              | 0.93                |
| $a_f$    | $\text{in}^2$ | 10.16             | 10.16               |
| $A_f$    | $\text{in}^2$ | 31.85             | 31.85               |

## Effective Geometric Properties: (cont.)

|       |                 | For Constant B.M. | For Sinusoidal B.M. |
|-------|-----------------|-------------------|---------------------|
| $e_f$ | in              | 15.34             | 15.34               |
| $I_f$ | in <sup>4</sup> | 1221              | 1221                |
| $S_f$ | in <sup>3</sup> | 79.6              | 79.6                |

## Computation Parameters

|                  |                    |                        |                        |
|------------------|--------------------|------------------------|------------------------|
| B                | [Eq.E-1]           | $1.146 \times 10^{-4}$ | $1.106 \times 10^{-4}$ |
| C                | [Eq.E-2]           | $3.923 \times 10^{-4}$ | $4.026 \times 10^{-4}$ |
| J                | [Eq.E-7]           | $2.996 \times 10^{-2}$ | $3.021 \times 10^{-2}$ |
| k                |                    | $6.50 \times 10^2$     | $3.40 \times 10^4$     |
| $\omega$         | [Eq.E-5]           | $6.663 \times 10^{-3}$ | $1.792 \times 10^{-2}$ |
| $2\lambda\omega$ |                    | 1.76                   | 4.73                   |
| n                | [Eq.E-6]           | 0.049                  | 0.358                  |
| $\delta$         | [Eq.E-3]           |                        | 3.657                  |
| $\theta$         | [Eq.E-4]           |                        | 1.921                  |
| $\psi$           | [Fig. 7<br>Ref.10] | 0.112                  | 0.879                  |
| $\phi$           | [Fig. 8<br>Ref.10] |                        | 0.091                  |

Therefore the stresses at the midlength of the deckhouse are:

a) for constant B.M. alone:

|                             | $k = 6.50 \times 10^2$ psi | $k = 3.40 \times 10^4$ psi |
|-----------------------------|----------------------------|----------------------------|
| $p_f [= -C\psi m]$          | -197 psi                   | -1545 psi                  |
| $p_1$ [Eq.E-15]             | +125 "                     | +978 "                     |
| $\bar{\sigma}_1$ [Eq.E-11]  | -4385 "                    | -1266 "                    |
| $\bar{\sigma}_f$ [Eq.E-12]  | -4385 "                    | -1266 "                    |
| $\sigma_1$ (keel) [Eq.E-13] | +4179 "                    | +2995 "                    |
| $\sigma_f$ (top) [Eq.E-14]  | +1704 "                    | -1672 "                    |

b) for sinusoidal B.M. alone:

|                         | $k = 6.50 \times 10^2 \text{ psi}$ | $k = 3.40 \times 10^4 \text{ psi}$ |
|-------------------------|------------------------------------|------------------------------------|
| $P_f$                   | -669 psi                           | -1798 psi                          |
| $P_1$                   | +480 "                             | +1288 "                            |
| $\bar{\sigma}_1$        | -4097 "                            | -1069 "                            |
| $\bar{\sigma}_f$        | -3319 "                            | -866 "                             |
| $\sigma_1(\text{keel})$ | +4621 "                            | +3421 "                            |
| $\sigma_f(\text{top})$  | +534 "                             | -2221 "                            |

Superimposing (a) and (b), we obtain the total stresses for the combination bending moment:

|                         |           |           |
|-------------------------|-----------|-----------|
| $\bar{\sigma}_1$        | -8482 psi | -2335 psi |
| $\bar{\sigma}_f$        | -7704 "   | -2132 "   |
| $\sigma_1(\text{keel})$ | +8800 "   | +6416 "   |
| $\sigma_f(\text{top})$  | +2238 "   | -3893 "   |

Above results are plotted in Figure 28.

Equations for Schade's Method Using  
"n" Terms of Moment Expansion

$$r_n = \frac{1}{2} \frac{1}{\cosh \frac{n\pi b}{2\lambda}} \left[ \frac{n\pi y}{2\lambda} \sinh \frac{n\pi y}{2\lambda} + 2 \cosh \frac{n\pi y}{2\lambda} - \frac{n\pi b}{2\lambda} \cosh \frac{n\pi y}{2\lambda} \tanh \frac{n\pi b}{2\lambda} \right]$$

[E-8A]

$$M = m + \sum a_n \sin \frac{n\pi x}{2\lambda}$$

[E-9A]

$$p_f = \alpha_1 \cos \gamma x \cosh \beta x + \alpha_2 \cos \gamma x \sinh \beta x + \alpha_3 \sin \gamma x \cosh \beta x \\ + \alpha_4 \sin \gamma x \sinh \beta x - Cm - \sum C_n \frac{\theta_n}{\delta_n} a_n \sin \frac{n\pi x}{2\lambda}$$

[E-10A]

where,

$$\gamma = \omega \sqrt{1 - \eta}$$

$$\beta = \omega \sqrt{1 + \eta}$$

[E-16A]

$$\delta_n \equiv 1 + \frac{\eta}{\omega^2} \left( \frac{n\pi}{2\lambda} \right)^2 + \frac{1}{4\omega^2} \left( \frac{n\pi}{2\lambda} \right)^4$$

[E-3A]

and

$$\theta_n \equiv 1 + \frac{E}{r_n e_l + e_f} \left[ \frac{I_l e_f}{a_l G} + \frac{r_n I_f e_l}{a_f G} \right] \left( \frac{n\pi}{2\lambda} \right)^2$$

$$+ \frac{E}{r_n e_l + e_f} \frac{r_n I_f e_l}{k} \left( \frac{n\pi}{2\lambda} \right)^4$$

[E-4A]

all other nomenclature, parameters and equations remain unchanged.